

INDIAN ROADS CONGRESS



BRIDGE LOADINGS ROUND THE WORLD

NEW DELHI
1966

Price (including packing &
postage) : Rs. 3.00

ROAD BRIDGES

RAILWAY BRIDGES

HIGHWAYS

TUNNELS

CHATTERJEE & POLK

23, BRABOURNE ROAD, CALCUTTA-1.

Announce

THEIR COLLABORATION

WITH

THE J. G. WHITE ENGINEERING CORPORATION

ENGINEERS AND CONSTRUCTORS

80, BROAD ST., NEW YORK 4, N.Y.



RAILROADS

SUBWAY
SYSTEMS

COMMUTER LINES

TRAFFIC SURVEYS

FEASIBILITY REPORTS

ENGINEERING &
ECONOMIC
INVESTIGATIONS

PRELIMINARY DESIGNS

PREPARATION OF
SPECIFICATIONS

FINAL DESIGNS
AND
TENDER DOCUMENTS

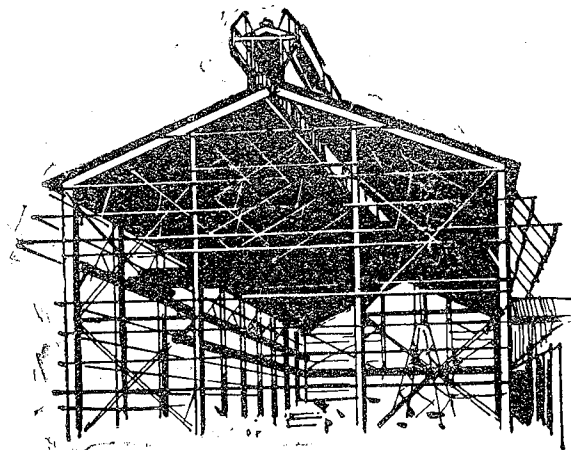
PROCUREMENT
AND
INSPECTION
OF MATERIALS

CONSTRUCTION
MANAGEMENT
AND SUPERVISION

ACHIEVEMENT • SERVICE • CAPABILITY

more than 75 years' experience now available

CP/2166



LIGHT AND MEDIUM STRUCTURAL FABRICATION

ROAD BRIDGES * STEEL FRAMES FOR
BUILDINGS * TEA GARDEN STRUCTURES
ROPEWAY TRESTLES & STATIONS
CONVEYORS & TOWERS * BUCKETS &
HANGERS * BUNKERS & CHUTES

Enquiries to :
STRUCTURAL DEPARTMENT

**MARTIN
BURN
LIMITED**



MARTIN BURN HOUSE,
12 Mission Row, Calcutta 1

MSDC-4

SHALIMAR TAR

Bridge Builders

In

Precast Prestressed Concrete

CURRENTLY COMPLETING THE LAST OF 6 BRIDGES
IN PURNEA OVER N. H. 34.

LAST YEAR COMPLETED TALLAH & MAJHERAT BRIDGES
IN CALCUTTA BOTH 9 MONTHS AHEAD OF SCHEDULE

SHALIMAR TAR PRODUCTS (1935) LIMITED

Regd. Office : 6 Lyons Range, Calcutta-I.

Phone : 22 23 81 (18 Lines)

GANNON DUNKERLEY & CO., LTD.

R. C. C. SPECIALISTS, ENGINEERS & CONTRACTORS

Regd. Office : CHARTERED BANK BUILDING, FORT, BOMBAY I.

Branches at : AHMEDABAD, COIMBATORE, CALCUTTA, MADRAS,

NEW DELHI, KANPUR, BANGALORE, POONA.



BRIDGE LOADINGS ROUND THE WORLD

The Indian Roads Congress issued a questionnaire to various countries in the world about the bridge loadings applied for design purposes.

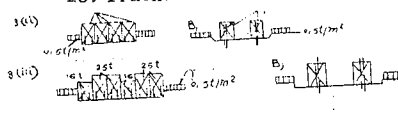
The following countries very kindly sent the replies:

- (1) America (U.S. Bureau of Public Roads)
- (2) Austria
- (3) Belgium—see pp. 128-129
- (4) Canada (Toronto and Ontario)
- (5) Finland
- (6) Germany (Federal Republic)
- (7) Great Britain
- (8) India
- (9) Italy
- (10) Japan
- (11) Malaysia
- (12) New South Wales (Australia)
- (13) New Zealand
- (14) Norway
- (15) Philippines
- (16) Rhodesia
- (17) Sweden
- (18) Switzerland
- (19) Turkey

The ensuing Tables have been prepared from the information received.

The Indian Roads Congress is very grateful to the above mentioned countries for the information supplied.

Transport-Communications Monthly Review

QUESTIONS	AMERICA (U.S. Bureau of Public Roads)	AUSTRIA			
		Bridge Classes	See Fig. 6 in Plate I		
1. LOADINGS	Article 1.2.5*	Total weight	tonnes	25	16
(i) Design train loading truck loading or any special loading for each category of roads.	(i) Truck loading and lane loading H10-44, H15-44, H20-44, H15-S12-44, H20-S16-44 and Military loading. (See Figs. 1, 2, 3 & 4 in Plate I). For trunk highways or other highways which carry heavy truck traffic, minimum loading H15-S12-44. For Inter-state highway system, Military loading (see Fig. 4 in Plate I) wherever it causes moments or shear greater than those caused by H20-S16 truck or the standard lane loading.	(a) Truck	tonnes	4	2.5
		Fore wheel	tonnes	8.5	5.5
		Back wheel	tonnes		
		Equivalent weight	tonnes/m ²	1.67	1.07
		(b) Uniform load	tonnes/m ²	0.50	0.40
		(c) Caterpillar			
		Car			
		Total weight	tonnes	60	—
		Caterpillar load	tonnes/m ²	17.14	—
		Equivalent weight	tonnes/m ²	3.33	—
		For spans more than 30 m, calculations may be made with the "Equivalent weight" (total weight, referred to the track area) instead of the different wheel loads.			
(ii) Distance between successive trains or loads	(ii) No successive trains or loadings.				
(a)					
(b)					
(c)					
(d)					
(e)					
2. Lateral disposition of train loading or other types of loadings with respect to:	Article 1.2.6*				
(a) Kerb	(a) Distance from kerb to centre line of wheel—2 ft (for slab design—1 ft). (see Figs. 1 and 3 in Plate I).	(a)	The standard trucks, 2.5 m wide, are equal to the width of a lane; therefore the trucks have to be put close together, so that the live load for the considered structural part arises to a maximum. It is not necessary to shift the wheels of the trucks to the kerb.		
(b) Central line of the bridge	(b) No particular lateral disposition.	(b)	The caterpillar, as the only load on the roadway, has a maximum deviation of 0.5 m on both sides from the centre line of the roadway.		
3. No. of train loadings taken for design of each span of bridges and culverts with the following number of traffic lanes:	Articles 1.2.7, 1.2.8, 1.2.9*		Bridge Class I: The calculation has to be executed for: (A) on 2 adjoining lanes, 1 truck of 25 t each. On every other of the following lanes, 1 truck of 16 t. The rest of roadway and footpath to be covered with uniformly distributed load of 0.5 t per metre. (B) For a caterpillar of 60 tonnes only, also see 2(b) above.		
(i) Two lanes	(i) 100 per cent of either 2-lane loading or one standard H or H.S. truck per each lane.		Bridge Class II: On 2 adjoining lanes, one 16 t truck each. The rest of the roadway and footpath to be covered with uniformly distributed load of 0.4 t per sq. metre.		
(ii) Two lanes each way on a divided highway	(ii) 75 per cent of either 4 lane loading or one standard H or H.S. truck per each lane.	3 (ii)	25t Trucks		
(iii) Three lanes each way on a divided highway	(iii) 75 per cent of either 6 lane loading or one standard H or H.S. truck per each lane.	3 (iii)	60 t CATERPILLAR		
	Note: In case of a 3-lane bridge, 90 per cent of the full live load.				

*Articles referred to relate to the A.A.S. H.O. Standard Specifications for Highway Bridges (1961).

CANADA

TORONTO

Truck loading is based on the H20-S16-44 or the heavier H25-S20 loading in the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

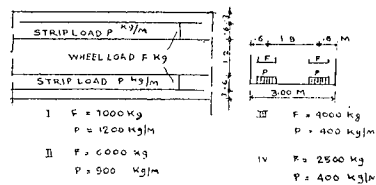
ONTARIO

All H20-S16-44

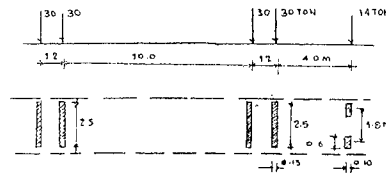
Article 1.2.4 to 1.2.9*

FINLAND

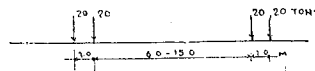
LANE LOAD



HEAVY TRUCK LOAD I



HEAVY TRUCK LOAD II

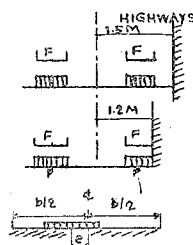


TORONTO

- (a) For trucks, wheel is placed 2 ft away.
(b) No relationship for trucks.

ONTARIO

Article 1.2.6.*



With Impact

Without Impact

For Heavy Truck Load $e \leq 0.1 b$

TORONTO

- (i) 2 train loads.
(ii) (a) 2 train loads.
(b) 3 train loads with 10 per cent reduction.
(c) 4 train loads with 25 per cent reduction whichever gives the most critical stress.
(iii) As in (i) above except that (c) becomes 4 lanes or more with 25 per cent reduction.

ONTARIO

Article 1.2.9*

- (i) 100 per cent
(ii) Two lanes 100 per cent, remaining lanes 50 per cent.

*Articles referred to relate to the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

number 1965

KEY

ways are
3. :
se, and
local and

ck

. m.
y loads of

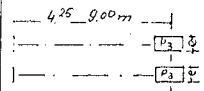
y loads of

design of

aviest of
by one or
be 1) with
2 (Type 3)

e of the

aded with
of trucks
loading on
)
n rollers
side with
footpaths

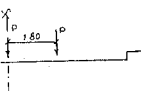


P_1 P_2 e
(t) (t) (cm)

8 8 51

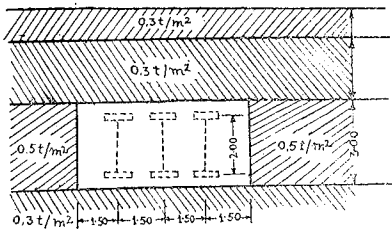
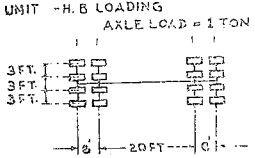
5 6 6 38

4 0 25



each lane.

k Code

QUESTIONS	FEDERAL REPUBLIC OF GERMANY	GREAT BRITAIN	Tran
<p>1. LOADINGS</p> <p>(i) Design train loading truck loading or any special loading for each category of roads.</p> <p>(ii) Distance between successive trains or loads</p> <p>(a)</p> <p>(b)</p> <p>(c)</p> <p>(d)</p> <p>(e)</p>	 <p>(i) For Federal Autobahns, Federal Highways and Rural Highways of 1st order, Bridge Class 60=6×10 t</p> <p>For Rural Highways of 2nd order and District Roads, Bridge Class 30=6×5 t</p> <p>For Minor Roads, Bridge Class 16 & 6, see Fig. 7 in Plate II.</p>	<p>Bridges carrying public roads are designed for HA loading (British Standard 153-Part 3-Section A—see Fig. 8 in Plate II consisting of a uniformly distributed load plus a single knife edge load. The U.D. load varies with the span but between 20 and 75 ft it is constant at 2200 lb per linear foot of 10 ft wide lane. The knife edge load is placed parallel to the supports of the member under consideration and has a value of 27,000 lb for a 10 ft wide lane.</p> <p>Bridges carrying important roads are checked for 45 units of HB loading to British Standard 153.</p> 	<p>Claus</p> <p>(i) Cl</p> <p>Cl</p> <p>Cl</p> <p>Cl</p> <p>(Se</p> <p>(a)</p>
<p>2. Lateral disposition of train loading or other types of loadings with respect to :</p> <p>(a) Kerb</p> <p>(b) Central line of the bridge</p>	<p>(a) For main carriageway, the load should be so placed as to cause worst effects. Laterally it can be placed upto the kerb.</p> <p>(b) Outside the carriageway, uniformly distributed load of 0.3 t per m² for bridges of class 60 and 30.</p>	<p>(a) Any part of the bridge deck between kerbs, including the hard shoulder, if any, is deemed to be subject to HA or HB loading.</p>	<p>(b)</p> <p>(c)</p> <p>(ii) See</p> <p>For</p> <p>For</p> <p>For</p>
<p>3. No. of train loadings taken for design of each span of bridges and culverts with the following number of traffic lanes :</p> <p>(i) Two lanes</p> <p>(ii) Two lanes each way on a divided highway</p> <p>(iii) Three lanes each way on a divided highway</p>	<p>(i) & (ii) The portion of train loading according relieving effect will not be considered.</p>	<p>(i) Two lanes of HA loading or one lane of HB loading together with one lane of 1/3 HA loading.</p> <p>(ii) Two lanes of HIA loading and two lanes of 1/3 HA loading or one lane of HB loading and the adjacent lane with 1/3 HA loading. The two lanes in the other carriageway each with HA loading.</p> <p>(iii) Two lanes of HA loading and four lanes of 1/3 HA loading or one lane of HB loading and the two adjacent lanes with 1/3 HA loading. In the other carriageway, two lanes with HA loading and the other lane with 1/3 HA loading.</p>	<p>Clause 2</p> <p>(a) Class</p> <p>kerb</p> <p>track</p> <p>12 ft</p> <p>bridg</p> <p>way;</p> <p>way:</p> <p>In ca</p> <p>6 in. 1</p> <p>20 in</p> <p>width</p> <p>having</p> <p>(b) Not g</p> <p>Clauses 11</p> <p>(i) One t</p> <p>wheel</p> <p>"A" t</p> <p>Highw</p> <p>(ii) 80 per</p> <p>"AA"</p> <p>or of</p> <p>train o</p> <p>(iii) This ty</p> <p>not per</p>

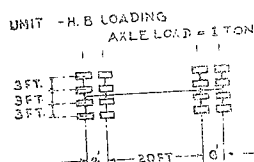
*Articles referred to relate

*Clauses of Pract.

GREAT BRITAIN

bridges carrying public roads are designed for HA loading (British Standard Part 3-Section A—see Fig. 8 in Plate II) consisting of a uniformly distributed load of a single knife edge load. The U.D. varies with the span but between 20 ft it is constant at 2200 lb per linear ft of 10 ft wide lane. The knife edge is placed parallel to the supports of the beam under consideration and has a value of 27,000 lb for a 10 ft wide lane.

bridges carrying important roads are designed for 45 units of HB loading to BS Standard 153.



Any part of the bridge deck between kerbs, including the hard shoulder, if any, is deemed to be subject to HA or HB loading.

Two lanes of HA loading or one lane of HB loading together with one lane of 1/3 HA loading.

Two lanes of HIA loading and two lanes of 1/3 HA loading or one lane of HB loading and the adjacent lane with 1/3 HA loading. The two lanes in the other carriageway each with HA loading.

Two lanes of HA loading and four lanes of 1/3 HA loading or one lane of HB loading and the two adjacent lanes with 1/3 HA loading. In the other carriageway, two lanes with HA loading and the other lane with 1/3 HA loading.

INDIA

Clauses 201* and 207*

- (i) Class "AA" tracked vehicle and Class "AA" wheeled vehicle.
Class "A" train of vehicles.
Class "B" train of vehicles.
(See Figs. 9, 10 & 11 in Plate III)

(a) For every 2-lane width of bridge Class "AA" one tracked or wheeled vehicle or two lanes of Class "A" loading whichever creates worst effects, to be used for road bridges in municipal limits, industrial and other specified areas and on specified highways.

(b) Class "A" to be used for all roads on which permanent bridges and culverts are constructed.

(c) Class "B" for temporary bridges, etc.

- (ii) See Figs. 9, 10 & 11 in Plate III

For Class "AA" tracked vehicles—
300 ft min.

For Class "A" train of vehicles—
65 ft min.

For Class "B" train of vehicles—
65 ft min.

Clause 207* (See Figs. 9 to 11, Plate III)

- (a) Class "AA" clear distance between kerb and outer edge of wheel or track = 1 ft for single lane bridge with 12 ft carriageway; 2 ft for multi-lane bridge with less than 18 ft carriageway; 4 ft for 18 ft or above carriageway.
In case of Class "A"—clear distance 6 in. between kerb and wheel having 20 in. width for all carriageway widths. Class "B" 6 in. for wheel having 15 in. width.

(b) Not given.

Clauses 113* and 208.2*

- (i) One train of Class "AA" tracked or wheeled vehicles or two lanes of Class "A" train of vehicles for National Highways and State Highways.

(ii) 80 per cent of two trains of Class "AA" tracked or wheeled vehicles or of four lanes of Class "A" train of vehicles.

(iii) This type of bridge construction is not permitted.

ITALY

For the purpose of loading, highways are divided into the following two categories.:

- (I) Highways for civil and military use, and
(II) Highways for civil use only (local and minor roads).

Loading system.

Type 1, a continuous train of 12 t truck

„ 2, a single 18 t steam roller

„ 3, a crowd load of 400 kg per sq. m.

„ 4, a continuous train of military loads of 61.5 t

„ 5, a continuous train of military loads of 32 t

„ 6, a single military load of 74.5 t

(See Fig. 13 in Plate—IV)

Loading system to be adopted in the design of highway bridges.

Category (I)—One military type—the heaviest of types 4, 5 or 6 flanked by one or more trains of truck (Type 1) with crowd loading of 400 kg/m² (Type 3) on the footpaths.

Category (II)—The most unfavourable of the following conditions:

(a) one or more lanes loaded with continuous trains of trucks (Type 1) with crowd loading on the footpaths (Type 3)

(b) one or more steam rollers (Type 2) side by side with crowd loading on footpaths (Type 3).

No details given.

No details given.

4.25 9.00

P₁ P₂
(t) (t)

8 8

5 6 6

4 0



each lane.

*Clauses referred to relate to the Indian Roads Congress Standard Specifications & Code of Practice for Road Bridges—Sections I & II (1964).

*Articles referred

December 1965

TODIESIA

ading, suppl
l with 30 uni
ading allowin
cent increas
nissible worl
esses.

Fig. 8
II for H
g. For H
3, see sket
Great Britai

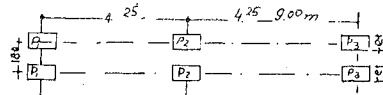
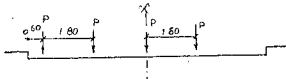
1. accordan
S. 153/195
—Section A.

" from kerb
analysis not
ting a lat
tion analysis
ntal loading
wheel is in
at edge of p
owing 25%
in permis
stresses.
mmetrical,
lateral distr
alysis is un

ull HA lo
g in each la
s above.

/A.

hree lanes.
A loading
adjacent la
HA in
d lane.

SWEDEN	SWITZERLAND	TURKEY																				
<p>See Fig. 15 in Plate IV</p> <p>(a) Lane loading consisting of one 14 t axle load + distributed "p" t/m, when $p=2.4$ t/m for loaded length less than 10 m, and 1.1 t/m for loaded length over 90 m.</p> <p>For loaded length between 10 m and 90 m, "p" varies according to formula</p> $p=2.4-\frac{1.3}{80}(l-10)$ <p>where l is the loaded length in metres (distance between the zero points of influence curve) or by interrupted loading, the sum of loaded lengths and length of the unloaded parts between.</p> <p>(b) Single truck loading of 100 t. This single truck loading may be assumed as exceptional loading without concurrent loading stress increased by 15 per cent.</p> <p>On special roads, it can be prescribed that the above mentioned single truck shall be considered as normal traffic loading.</p>	<p>(i) Main roads : Distributed load of 360 kg per m^2 and one axle load of 15 t for each lane.</p> <p>Secondary roads : The same with 240 kg per m^2 and 10 t.</p> <p>(ii) Instead of distributed loading with one axle load, all parts should be checked also for the following train load :</p> <p>3 axle loads for each lane with 1.5 m distance behind each other of 15 t for main roads, 10 t for secondary roads.</p>	<div></div> <table><thead><tr><th></th><th>P_1 (t)</th><th>P_2 (t)</th><th>P_3 (t)</th><th>e (cm)</th></tr></thead><tbody><tr><td>State Roads H20-S16</td><td>2</td><td>8</td><td>8</td><td>51</td></tr><tr><td>Provincial Roads H15-S12</td><td>1.5</td><td>6</td><td>6</td><td>38</td></tr><tr><td>Village Roads H10</td><td>1</td><td>4</td><td>0</td><td>25</td></tr></tbody></table>		P_1 (t)	P_2 (t)	P_3 (t)	e (cm)	State Roads H20-S16	2	8	8	51	Provincial Roads H15-S12	1.5	6	6	38	Village Roads H10	1	4	0	25
	P_1 (t)	P_2 (t)	P_3 (t)	e (cm)																		
State Roads H20-S16	2	8	8	51																		
Provincial Roads H15-S12	1.5	6	6	38																		
Village Roads H10	1	4	0	25																		
<p>See Fig. 15 in Plate IV.</p>	<p>The distributed load goes all over the bridge. The train and axle loads in the position which gives the maximum stress.</p>	<div></div>																				
<p>(i) Two lane loading or single truck loading.</p> <p>(ii) Each side of the divided highway shall essentially be regarded as belonging to a separate bridge. However, in some cases on long bridges, certain deduction in the loading is often allowed.</p>	<p>(i) Not more as two lanes are loaded with axle or train loads.</p> <p>(ii) & (iii) Not more as two lanes each way have to be loaded with axle or train loads.</p>	<p>One truck for each lane.</p>																				

QUESTIONS	AMERICA (U.S. Bureau of Public Roads)	AUSTRIA	CANADA
4. Area of the train of vehicle assumed in elevation for calculating the effective wind pressure.	Article 1.2.14* Wind pressure at the rate of 100 lb per linear ft on moving live load acting at 6 ft above deck. This is to be taken only for group loading combination as explained in Article 1.4.1*.	The Austrian standards adopt a 2.5 m high rectangular traffic area in correspondence with the position of live load.	TORONTO A simplified procedure is used for spans 125 ft and under. 100 lb per linear ft transversely. 40 lb per linear ft longitudinally. Both forces applied simultaneously 6 ft above deck. ONTARIO Article 1.2.14*
5. Impact factor due to live loads assumed for different types of loads on : (i) Concrete bridges 			

*Articles referred to relate to the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

	FINLAND	FEDERAL REPUBLIC OF GERMANY	GREAT BRITAIN	TURKEY
to be re.	Height 2 m for the length of loading.	For bridges without load—250 kg/m ² For bridges under construction—125 kg/m ² For bridges with load—125 kg/m ² For pedestrian & cycle bridges—75 kg/m ² The above loading acting in case of (a) road bridges at 2.0 m height (b) pedestrian bridges at 1.8 m height.	A plane with a continuous height of 8 ft above the carriageway for highway bridges or 4 ft above the footway for foot-bridges. Allowance may be made for the screening effect of the structure on the plane, based on projected areas.	Effective wind pressure is calculated as follows: for unloaded bridge : 5 times the height of the full area of the ridge deck including the hand-rails. Load 50 kg per m ² . for loaded bridges : eight of the area of the bridge deck elevation + 2.0 m for live load.
6 in.	I=40 per cent when filling h(m) ≥ 3 m I=16 (3.0-h) per cent for height of fill varying from 0.5-3.0m For timber bridges always I=20 per cent.	Impact Factor $\phi = 1.4 - 0.008 \times l\phi \geq 1.0$ $l\phi$ = governing length in metres (See D.I.N. 1073, 1074, 1075 and 1078 for the determination of the value of $l\phi$).	HB loading has no impact factor. HA loading incorporates an impact factor of 25 per cent on the heaviest axle in the train of vehicles from which HA loading is derived. It is constant for all forms of construction.	Wind load 125 kg per m ² .
to ft.				$\phi = 1 + \frac{15}{L + 37}$ here L = span length in metres. $\phi_{\max.} = 1.3$
(W)				
ft.)				
de-				
rel				
lied				
ce				

QUESTIONS

4. Area of the train of vehicle assumed in elevation for calculating the effective wind pressure.

NEW SOUTH WALES

Clause 2.15*

Lateral wind force at 100 lb per linear ft acting at a height of 6 ft above the deck.

Alternatively, 66 lb per linear foot laterally plus 33 lb per linear foot longitudinally acting simultaneously.

NEW ZEALAND

Wind loads as per A.A.S.H.O. Specifications for longitudinal elevation.

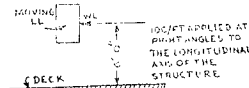
See Fig. 3 in Plate I and sketch in Q. 1, p. 101.

NORWAY

Normally no wind pressure is considered on loaded bridge.

On bridge without load, wind pressure is assumed equal to 250 kg per m² of exposed area.

PHILIPPINES



See Figs. 1, 2 and 3 in Plate I.

q=

q

F

P

See

M_i

(i)

5. Impact factor due to live loads assumed for different types of loads on:

- (i) Concrete bridges

- (ii) Steel bridges

- (iii) Prestressed concrete bridges

Variation of impact according to span length.

Clause 2.13*

- (a) Impact = 10 per cent for steel or concrete substructure above the foundations but not rigidly connected to the superstructure and structures carrying 1½ to 3 ft of fill.

- (b) Steel or concrete superstructures and those parts of steel or concrete substructure above the foundations which are rigidly connected to the superstructure as in the rigid frames or continuous designs and structures carrying less than 1½ ft of fill. The impact shall be

$$I = \frac{5000}{L+125} \text{ per cent}$$

(max. 30 per cent)

(min. 10 per cent)

Impact factor not dependent on bridge type. Impact factor in use:

$$I = \left(\frac{50}{L+125} \right) \times 100 \text{ per cent}$$

Impact is included under Q. 1—equivalent loading.

The local loading (Q. 2) assumes 5 t impact for an allowable 13 t axle load (38.5 per cent)

Allowable gross weights of vehicles are established by comparing their effect with the effect of the class II equivalent loading.

By this, the above 38.5 per cent impact is added to the heaviest axle, but it is so far considered unnecessary to add impact to the remaining axles.

For all kinds of bridges,

$$I = \frac{50}{L+125} \text{ in which}$$

I = impact fraction (maximum 30 per cent)

L = length in feet of the portion of the span which is loaded to produce the maximum stress in the member.

*Articles referred to

*Clauses referred to relate to Highway Bridge Design Specifications of N.A. of A.S. Road Authorities (1965).

RHODESIA	SWEDEN	SWITZERLAND	TURKEY
As under Section 12— B.S. 153/1954—Part 3, Section "A".	The wind pressure area of traffic load shall be supposed to be a rectangle 2 metre higher from the deck and length equal to loaded length.	For road bridges, 3 metre high band is assumed.	Effective wind pressure is calculated as follows : (a) for unloaded bridge : 1.5 times the height of the full area of the bridge deck including the hand-rails. Load 250 kg per m ² . (b) for loaded bridges : height of the area of the bridge deck elevation + 2.0 m for live load. Wind load 125 kg per m ² .
<p>(i) & (ii) Concrete and steel bridges—Included in HA loading. See B.S. 153/1954.</p> <p>(iii) For Prestressed concrete bridges : As above, but for dynamic stability the following apply :</p> <p>(a) The vertical acceleration of the superstructure under $\frac{1}{2}$ HA loading, travelling at 40 m.p.h. shall not exceed 0.5 ft per sec².</p> <p>(b) The natural frequency of the superstructure under a live load of 100 lb per sq. ft. shall not be less than 3 cycles/sec., where the natural frequency equals</p> $0.75 \sqrt{\frac{EI}{wL^4}} \text{ cycles/sec.}$ <p>where "E" is Young's Modulus for the superstructure in lb per sq. in., "I" moment of inertia of superstructure (in.⁴). $w = B \times 100 +$ weight of superstructure in lb/ft run. "B" = breadth of superstructure. "L" = span of superstructure in feet.</p>	<p>40 per cent for lane loading and only for the wheel loads, and not for the uniform load p.</p> <p>When wheel is in contact with railing or another limitation (kerb, etc.) of the roadway, no impact allowance has to be considered.</p> <p>No impact for single truck loading.</p>	<p>Impact = $5 \times \frac{100 + L}{10 + L}$</p> <p>where L is the length of span in m.</p>	<p>$\phi = 1 + \frac{15}{L + 37}$</p> <p>where L = span length in metres. $\phi_{\text{max.}} = 1.3$</p>

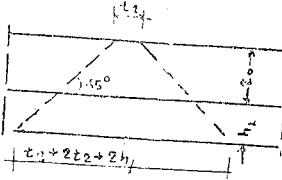
Transport-Communications Monthly Review

QUESTIONS	AMERICA (U.S. Bureau of Public Roads)	AUSTRIA	CANADA
<p>6. Ground Contact Area</p> <p>The shape of contact area for design calculations and formulae adopted for dispersion of the wheel loads through the wearing coat and the slab for designing.</p>	<p>Article 1.2.6 and 1.3.2 (c)</p> <p>As per Figs. 1 and 3 in Plate I.</p> <p>For further details, see Article 1.3.2 (c).</p>	<p>For the shape of the contact area, see Q. 1, Loadings. If there is a load distributing layer, concentrated loads may be dispersed under an angle of 45 degrees. The dispersion may be extended to the centroidal axis of the considered structural part only.</p>	<p>TORONTO</p> <p>Shape is circular for slabs on soil. Otherwise dispersion is in accordance with "Standard Specifications for Highway Bridges" (A.A.S.H.O.—1961).</p> <p>ONTARIO</p> <p>Article 1.3.2*</p>
<p>7. Equivalent UDL or knife edge loading, if adopted for working out :</p> <p>(i) Bending moment.</p> <p>(ii) Shear.</p>	<p>Articles 1.2.7 and 1.2.8*</p> <p>See Fig. 2 in Plate I.</p>		<p>TORONTO</p> <p>(i) For bending moment 640 lb per linear ft UDL+18000 lb (in lieu of H20—S16 truck)</p> <p>(ii) For shear 640 lb per linear ft UDL+26000 lb (in lieu of H20—S16 truck)</p> <p>ONTARIO</p> <p>None</p>
<p>8. The percentage of the live load on the bridge taken for calculating the braking force in the design of substructure of the bridge.</p>	<p>Article 1.2.13*</p> <p>5 per cent of L.L. without impact in all lanes carrying traffic headed in the same direction acting at 6 ft above deck. The load shall be lane-load plus knife edge load without impact and reduction as per Q. 3 applied. Longitudinal forces due to friction of beams shall also be provided for in the design.</p>	<p>30 per cent of the weight of the heaviest vehicle (caterpillars excluded).</p>	<p>TORONTO</p> <p>5 per cent</p> <p>ONTARIO</p> <p>Article 1.2.13*</p>

*Articles referred to relate to the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

Transport-Communications Monthly Review

December 1965

FINLAND	FEDERAL REPUBLIC OF GERMANY	GREAT BRITAIN
	<p>For ground contact area of wheel load, see Fig. 7 in Plate II.</p> <p>Generally the dispersion of wheel load be taken at 45 degrees. In the case of massive slabs, the dispersion as above will be up to the middle of the slab.</p>	<p>The contact area for a heavy wheel of 11½ tons in the HB loading is taken as 15 in. x 3 in. with the 3 in. in the direction of travel. This load may be dispersed through the wearing course and slab at an angle of 45 degrees longitudinally and transversely. For structural distribution in a slab, normal structural theory may be used, e.g., Pigeaud or Westergaard.</p>
See Q. 1	See Q. 1.	
20 per cent	100 per cent	<p>The longitudinal force for HA loading is 10 Tons for spans up to 10 ft, plus 0.5 Ton per foot of span over 10 ft with a maximum of 25 Tons. The longitudinal force for 45 units of HB loading is 45 Tons for all spans.</p>

KEY

ient perpendicular to

rom 0.6 to 2.0 m

+0.76

n $E=0.4S+1.14$

ment parallel to traffic

75S+0.98



ae :

(q t/m)	Q(t)	
	Moment	shear
1.00	9.00	13.00
0.75	6.75	9.75
0.50	4.50	6.50

nt of one standard
the whole width
idge applied on
ice of the deck.

Transport-Communications Monthly Review

ITALY

Front Wheel

Rear Wheel

QUESTIONS		Transport-Communications Monthly Review	
		INDIA	ITALY
6. Ground Contact Area	A P F A	<p>Clauses 207 and 207-2.* See Figs. 9, 10 and 11 in Plate III, for shape of contact areas. For one way slab dispersion=effective width measured parallel to the supported edges :</p> $e=kx \left(1-\frac{z}{l} \right)+W \text{ and }$ $e=1.2x+W \text{ for cantilever.}$ <p>Effective dispersed length of slab (in the direction of span)= dimension of tyre contact area in the direction of span+twice the thickness of slab and wearing coat. For two way slabs, dispersion as per rational method.</p>	<p>In slab calculations, the weight of a rear wheel (6t) of the roller is distributed over a rectangle, one side of which is equal to the sum of the width of the roller wheel plus twice the depth of slab and wearing coat; the other side is equal to 10 cm plus twice the depth of slab and wearing coat.</p> <p>For the bridge on highway of category I, an additional calculation must be made in respect of the two rear axles, each of 18t, of load in Type 6. In such a case, the total load of the two axles must be distributed over a rectangle with sides of $2.65 \times 1.12\text{m}$ each side being increased by twice the depth of slab and wearing coat.</p> <p>Normally the calculation is carried out for a slab. When the lengths of the sides differ substantially, the slab can be considered as being bound by the longer sides, increasing the rectangular distribution in the direction of those sides by one half of the shorter side.</p> <p>On main beams</p> <p>In considering transverse distribution, the loading should be so placed as to give the most unfavourable effects.</p> <p>In the case of beam and slab construction, if a rigorous calculation is not made based on the theory of slabs, then all the beams should be similar and designed to carry the increased edge stresses.</p>
7. Equivalent UDL or knife edge loading, if adopted for working out :	Arti. Sec :	Nil	See Table 2 page 131
(i) Bending moment.			
(ii) Shear.			
8. The percentage of the live load on the bridge taken for calculating the braking force in the design of substructure of the bridge.	Article 5 per cent out im. carryin in the acting The lo. load pl without reducti applied forces c beams s vided f	<p>Clause 214-2*</p> <p>(a) 20 per cent of the first train load plus 10 per cent of the loads of succeeding trains or part thereof, the train loads in one lane only being considered for this purpose. When only part of the first train is on the full span, the braking force shall be only 20 per cent of portion of load on the span.</p> <p>(b) For bridges having more than two lanes : as in (a) above for the first two lanes plus 5 per cent of the loads on the lanes in excess of two. (Effect of impact is not taken into consideration).</p>	<p>Braking force shall be equal to 1/10 of the load superimposed by a continuous train of trucks (Type 1). This force, however, shall not be less than 0.3 of the heaviest axle of the load system being considered.</p>
*Articles referred to relate to the		referred to relate to the Indian Roads Congress Standard Specifications Sections I and II (1964).	

*Articles referred to relate to the


*Clauses referred to relate to the Indian Roads Congress Standard Specifications and Code of Practice for Road Bridges—Sections I and II (1964).


ght of a rear wheel
d over a rectangle,
e sum of the width
the depth of slab
e is equal to 10 cm
d wearing coat.
ay of category I, an
e made in respect of
t, of load in Type 6.
oad of the two axles
rectangle with sides of
increased by twice
coat.
carried out for a slab
s differ substantially,
being bound by the
rectangular distribu-
sides by one half of

Distribution, the load-
s to give the most

1 slab construction, if
ot made based on the
beams should be simi-
the increased edge

JAPAN

Front Wheel  12.5 cm
20 cm

Rear Wheel  50 cm
20 cm

For bridges
with span length
less than 150 m,
it is not adopted.
For longer spans,
equivalent U.D.L
may be specified.

10 per cent of T loading.

equal to 1/10 of the load
tinuous train of trucks
however, shall not be less
t axle of the load system

MALAYSIA

B.S. 153 : Part 3A : 1954

Appendix A 1(c) and 3(f)

Contact area of 15 in. x 3 in.,
the smaller dimension being in
the direction of travel.

Dispersal under the wheel load
shall be taken at 45 degrees.

B.S. 153 : Part 3A : 1954
Appendix A.

See Fig. 8 in Plate II.

B.S. 153 : Part 3A : 1954
Clause 10

Span upto 10 ft = 10 Tons

Span above 10 ft = 10 Tons +
 $\frac{1}{2}$ Ton for each
ft of span over
10 ft, but not
exceeding 25
Tons.

TURKEY

nforcement perpendicular to

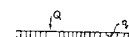
1 (S) from 0.6 to 2.0 m

= 0.6S + 0.76

an 2 m E = 0.4S + 1.14

nforcement parallel to traffic

= 0.175S + 0.98



ne :

(q t/m)	Q(t)	
1.00	Moment	shear
0.75	9.00	13.00
0.50	6.75	9.75
	4.50	6.50

nt of one standard
the whole width
idge applied on
ice of the deck.

Transport-Communications Monthly Review

QUESTIONS		Transport	NEW SOUTH WALES	NEW ZEALAND	NORWAY	PHILIPPINE	RH
6.	Ground Contact Area	A A P F A	<p>Clause 2.5*</p> <p>See Figs. 1 & 3 in Plate I.</p> <p>Contact width of each rear tyre equals 1 inch per every 2000 lb of total weight of loaded truck.</p>	<p>The shape of contact area as per A.A.S.H.O. H20-S16-44 loading.</p> <p>Distribution according to the A.A.S.H.O. Specification based on Westergaard method.</p>	<p>The contact area for the wheel load given under Q. 2 is 50 cm lateral, by 20 cm in driving direction.</p> <p>For moment calculation, the above area is increased by thickness of wearing coat + 50 per cent of slab thickness in each direction.</p>	<p>See Figs. 1 and 2 in Plate I and Article 1.3.2 (c) A.A.S.H.O. Bridge Specifications (1961).</p>	<p>Elliptical axis 2 in. Pi dispersion degree area force: tural</p>
7.	Equivalent UDL or knife edge loading, if adopted for working out : (i) Bending moment. (ii) Shear.	Article See 1	<p>Clause 2.5*</p> <p>See Fig. 2 in Plate I</p> <p>Worked out bending moment and shear for various spans is given in Appendix A (pages 172-183) of NAASRA Highway Bridge Design Specifications (1965).</p>	<p>As per A.A.S.H.O. H20-S16-44 Lane Loading.</p>	<p>See Q. 2 above.</p> <p>For shear, the actual knife edge load "A" reaches the max. value of 16 tonnes according to formula</p> $\left(x = \frac{L}{2} \right)$	<p>Similar to A.A.S.H.O. Bridge Specifications (1961).</p> <p>Fig. 2 in Plate I.</p>	<p>(i) N (ii) I</p>
8.	The percentage of the live load on the bridge taken for calculating the braking force in the design of substructure of the bridge.	Article 5 per cent out im carryin in the acting The load pl without reductio applied forces d beams d vided	<p>Clause 2.14*</p> <p>5 per cent of total live load (without impact) on the bridge loaded to give maximum effect. This shall be taken as acting 6 ft above road level.</p>	<p>None in the case of road bridges.</p>	<p>So far braking force of 8 t for lane lengths upto 5 m increasing to 12 t for 25 m length or more has been adopted. At present the question of increasing braking forces is being considered.</p>	<p>5 per cent of the total lane loading for moment without impact and traffic headed in the same direction subject to reduction in the load intensity as follows :</p> <p>One or two lanes 100 per cent</p> <p>Three lanes 90 per cent</p> <p>Four lanes or more 75 per cent</p>	<p>See (195) "A"</p>

*Clauses referred to relate to Highway Bridge Design Specifications of N. A. of A. S. Road Authorities (1965).

*Articles referred to relate to

PHILIPPINES

See Figs. 1 and 2 in Plate I and also Article 1.3.2 (c) of A.A.S.H.O. Bridge Specifications (1961).

Similar to A.A.S.H.O. Bridge Specifications (1961).

Fig. 2 in Plate I.

per cent of the total lane loading or moment without impact and traffic loaded in the same direction subject to reduction in the load intensity as follows:

one or two lanes 100 per cent

three lanes 90 per cent

four lanes or more 75 per cent

of N. A. of A. S.

RHODESIA

Elliptical, major axis 21 in., minor 9 in. Pigeaud's general dispersion, i.e., 45 degrees from contact area to main reinforcement in structural member.

SWEDEN

Shape of contact area—See Fig. 15 in Plate IV.

See Fig. 15 in Plate IV.

(i) N/A

(ii) N/A

See para 10 B.S. 153 (1954) Part 3 Section "A".

Irrespective of the clear width of the roadway, the braking force shall be 7 tonnes for 20 m length & 12 tonnes per 30 m length or more uniformly distributed over the clear width of roadway. For intermediate lengths, linear interpolation is applied.

SWITZERLAND

—

—

—

TURKEY

Main reinforcement perpendicular to traffic :

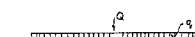
Slab span (S) from 0.6 to 2.0 m

$$E = 0.6S + 0.76$$

Bigger than 2 m $E = 0.4S + 1.14$

Main reinforcement parallel to traffic

$$E = 0.175S + 0.98$$



For each lane :

	(q t/m)	Q(t)	
		Moment	shear
H20-S16	1.00	9.00	13.00
H15-S12	0.75	6.75	9.75
H-10	0.50	4.50	6.50

15 per cent of one standard truck for the whole width of the bridge applied on the surface of the deck.

Transport-Communications Monthly Review

QUESTIONS	AMERICA (U.S. Bureau of Public Roads)	AUSTRIA	CANADA
9. The surcharge effect considered in the design of abutments of the bridge due to the live load on the approach fill.	Article 1.2.19* Surcharge effect on the abutment due to the live load on approach fill=2 ft. No surcharge, if adequately designed R.C.C. approach slab is provided.	No special standards.	TORONTO Equivalent to an additional 2 ft of backfill. ONTARIO 2 ft surcharge without approach slab. None if approach slab is used.
10. Footpath loading (state) min. width of footpath acceptable. (i) Crowd load assumed per sq. ft. (ii) Any variation in the above load for change in the span length. (iii) Any special loading specified for the accidental mounting of vehicles on the footpath and in that case overstressing, if any, allowed.	Article 1.2.11 (c)* No minimum width of footpath. (i) 85 lb per sq. ft. for slab, stringers and immediate supports. (ii) For girders, trusses, arches, etc. 0—25 ft span —85 lb/sq. ft. 26—100 ft span —60 lb/sb. ft. Over 100 ft span, . $P = \left[30 + \frac{3000}{L} \right] \left[\frac{5.5-W}{50} \right]$ where P = L.L. per sq. ft. (max. 60 lb per sq. ft.) L = loaded length of side-walk in feet. W = width of side-walk in ft. (iii) No such loading.	Minimum width = 1.50 m Bridge Class I = 0.5 t per m ² Bridge Class II = 0.4 t per m ² Nil One truck of 25 tonnes (16 tonnes) placed to produce maximum stress. No overstressing allowed.	TORONTO Normal use-100 lb per sq. ft. Nil Not definite. ONTARIO Article 1.2.11*

*Articles referred to relate to the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

	FINLAND	FEDERAL REPUBLIC OF GERMANY	GREAT BRITAIN	TURKEY
<p>DA</p> <p>n additional</p> <p>e without</p> <p>ach slab is</p>	<p>$\frac{1}{12}F + \frac{2}{3}P$</p> <p>n kg per m²</p> <p>n kg</p> <p>n kg per m</p> <p>2.1.</p>	<p>Earth surcharge and earth pressure are determined from the characteristic value of the soil at site.</p> <p>The traffic load considered in the design of bridge should be placed at the unfavourable positions.</p> <p>Individual loads can be substituted by uniformly distributed loading.</p>	<p>The surcharge effect is taken as being equivalent to two feet height of fill.</p>	<p>0.80 m extra height of earth fill.</p>
<p>0 lb per sq. ft.</p> <p>l</p> <p>e.</p> <p>.11*</p>	<p>mum width=1.5 m</p> <p>0 kg per m²</p> <p>—</p> <p>wheel load F (Sec</p> <p>c & reinf. 65 per cent</p> <p>Yield point</p>	<p>(i) 0.5 t per m² in case of spans less than 10 m</p> <p>(ii) $0.550 - 0.005L \geq 0.4$ t per m²</p> <p>L=span in metre</p> <p>(iii) No</p>	<p>80 lb per sq. ft.; minimum width normally accepted is 6 ft.</p> <p>100 lb per sq. ft.</p> <p>These loadings may be reduced in the same proportion as the uniformly distributed load in HA loading, for spans above 75 feet.</p> <p>A four ton wheel load occupying a 12-in. diameter circle. 50 per cent over-stress is permitted.</p>	<p>Minimum width—</p> <p>0.75 m</p> <p>360</p> <p>(i) 300 kg per m² (for spans upto 30 m)</p> <p>ls...</p> <p>(ii) $p = \left(0.3 + \frac{0.9}{L} \right) \left(5.5 - \frac{W}{3} \right)$</p> <p>(t per m²) for spans bigger than 30 m</p> <p>L=span length in m.</p> <p>W=footpath width in m</p> <p>oad</p> <p>g)</p>

(1961).

QUESTIONS	(U)	NEW ZEALAND	NORWAY	PHILIPPINES
9. The surcharge effect considered in the design of abutments of the bridge due to the live load on the approach fill.	Art Sur abu load ft. No at el app ed.	When highway traffic can come within a distance from the top of the structure equal to one half the height, the pressure shall have added to it a surcharge pressure equal to not less than 2 ft of filling.	Usually a surcharge of 2 t per sq. metre has been considered for abutment design. A differentiation between low and high abutments ought to be introduced.	2 ft L.L. surcharge added to earth press.
10. Footpath loading (state) min. width of footpath acceptable.	Arti No foot	Minimum width 4 ft	For design of footpath structures : 400 kg per m ²	Minimum width—2 clear.
(i) Crowd load assumed per sq. ft.	(i) slab mec	(i) 60 lb per sq. ft. (50 per cent of this when combined with main traffic live loads).	(i) Contemporary footpath loading and local load according to Q. 2 200 kg per m ² footpath.	(i) 85 lb per sq. ft.
(ii) Any variation in the above load for change in the span length.	(ii) 26 (P = [wher P = L 6 L = 1 s W =	(ii) No	(ii) Contemporary footpath loading and equivalent loading according to Q. 1 : 0.1 × P per sq. metre foot-path and not more than 200 kg per m ² .	(ii) Spans upto 0-85 lb per sq. ft. Spans from 2-100 ft—60 lb per sq. ft. Over 100 ft sp $P = \left(30 + \frac{3000}{L} \right)$ P = L.L. per sq. ft. (maximum 60 lb per sq. ft.) L = loaded length walk in ft W = width of side
(iii) Any special loading specified for the accidental mounting of vehicles on the footpath and in that case overstressing, if any, allowed.	(iii)	(iii) No allowance	(iii) Control for a "runway" wheel of 6.5t (without impact) placed with its contact area close to railing. 50 per cent overstressing permitted.	(iii) Concentrated load of 15600 one foot from of rail. Stress $f_s = 30000$ p $f_c = 1670$ p N = 10

*Articles referred to relate to t

See
B SCalcu
struct
Civil
Practi
Retain
issued
Struct
Lond

Minix

(i) 8

(ii) C
B
P(iii) A
a
ti
p
p
P
si

December

PHILIPPINES

ft L.L. surcharge
added to earth press

Minimum width—2
clear.

(i) 85 lb per sq. ft.

(ii) Spans upto 0-
85 lb per sq. ft.
Spans from 2
100 ft—60 lb p
Over 100 ft sp

$$P = \left(30 + \frac{3000}{L} \right) \left(\frac{W}{3} \right)$$

P=L.L. per sq. ft.
(maximum 60 lb p

L=loaded length
walk in ft

W=width of side

in-
5t
ced
rea
per
ing

(iii) Concentrated
load of 15600
one foot from
of rail.

Stress $f_s = 30000$ p
 $f_c = 1670$ p
N= 10

RHODESIA	SWEDEN	SWITZERLAND	TURKEY
<p>See clause 1.4 B S. 153/1954 Part 3</p> <p>Calculations of forces on structures, page 16 et. seq. Civil Engineering Code of Practice No. 2 (1951) "Earth Retaining Structures" issued by the Institution of Structural Engineers, London.</p>	<p>1.5 t per m² each lane of 3 m width. For more than 2 lanes, the surcharge may be reduced in the same proportion as the loading, i.e. for more than two lanes, only 50 per cent extra lanes is to be added. This surcharge may be considered uniformly dis- tributed over the width of abutment.</p>	<p>2 tonnes per m²</p>	<p>0.80 m extra height of earth fill.</p>
<p>Minimum width 2 ft 6 in.</p> <p>(i) 80 lb per sq. ft.</p> <p>(ii) Clause 4C, B.S. 153/1954 Part 3 Section A</p>	<p>Minimum width—1.5 m.</p> <p>Separate footpath</p> <p>(i) 400 kg per m² uniformly distributed when loaded length exceeds 10 m. The above mentioned load be reduced for main girders and arches to 1/6 pt. per m² P=uni- form lane loading as per Fig. 15 in Plate IV.</p> <p>(iii) Accidental loading of a 4 Ton wheel is inves- tigated at edge of parapet, allowing 25 per cent increase in permissible working stresses.</p>	<p>(i) Main roads..... 360 kg per m²</p> <p>Secondary roads... 240 kg per m²</p> <p>(ii) No</p> <p>(iii) Single wheel load of 6 tonnes (No over stressing)</p>	<p>Minimum width— 0.75 m</p> <p>(i) 300 kg per m² (for spans upto 30 m)</p> <p>(ii) $p = \left(0.3 + \frac{0.9}{L} \right) \left(5.5 - \frac{W}{3} \right)$ (t per m²) for spans bigger than 30 m L=span length in m. W=footpath width in m</p>

Transport-Communications Monthly Review

QUESTIONS	AMERICA (U.S. Bureau of Public Roads)	AUSTRIA	CANADA
<p>11. Foot bridges.</p> <p>(i) The minimum width of foot bridge acceptable.</p> <p>(ii) The loading specified in the design of the deck in</p> <p>(a) Urban areas</p> <p>(b) Rural areas</p> <p>(iii) Loading stipulated for the design of hand-rails.</p>	<p>Article 1.2.11*</p> <p>(i) No such minimum widths specified.</p> <p>(ii) No distinction made.</p> <p>(iii) See Fig. 5 in Plate I.</p>	<p>(i) No standard dimensions.</p> <p>(ii) Austrian standards assume no different loads for rural and urban areas.</p> <p>Bridge class I—uniform load of 0.5 per t m².</p> <p>Bridge class II—uniform load of 0.4 t per m².</p> <p>(iii) 0.08 t per m on the upper edge of the hand-rail in horizontal and vertical direction.</p>	<p>TORONTO</p> <p>(i) Variable</p> <p>(ii) Urban areas } 100 lb per and } sq. ft. Rural areas }</p> <p>(iii) No definite specification</p> <p>ONTARIO</p> <p>(i) As required</p> <p>(ii) Article 1.2.11*</p> <p>(iii) 1.2.11 revised interim (1964)</p>
<p>12. Any formulae stipulated for calculating the impact on piers and abutments due to floating objects in the river</p> <p>(i) floating timber</p> <p>(ii) vessels and small river craft</p> <p>(iii) ice</p>	<p>Article 1.2.17*</p> <p>(i) No details given.</p> <p>(ii) No details given.</p> <p>(iii) No details for impact of ice are given.</p> <p>However ice pressure on piers to be taken at 400 lb per sq. in. The thickness and height of ice to be determined by site investigation.</p>		<p>TORONTO</p> <p>(i) & (ii) None since Canada's climate dictates that ice is normally critical.</p> <p>(iii) Dictated by site conditions (i.e. size of river, water velocity etc.)</p> <p>ONTARIO</p> <p>Article 1.2.17*</p>
<p>13. Any other information supplied.</p>	<p>Clearance.</p> <p>For structures over Interstate Highway System—16 ft clear over the entire width of roadway including shoulders.</p>		

*Articles referred to relate to the A.A.S.H.O. Standard Specifications for Highway Bridges (1961).

FINLAND	FEDERAL REPUBLIC OF GERMANY	GREAT BRITAIN	TURKEY
(i) 4.0 m (ii) (a) 400 kg per m ² or 7 ton axle (b) —do— (iii) Uniformly distributed load 80 kg per m concentrated load 100 kg (vertical or horizontal).	(i) — (ii) No special specification	(i) 6 ft (ii) (a) 100 lb per sq. ft. (b) 80 lb per sq. ft. (iii) Between 50 lb and 100 lb per linear foot according to situation. The force to be applied 3 ft above the footway.	2.5 m a) 400 kg per m ²
			b) 250 kg per m ² 100 kg per m
(i) protection required (ii) 1.0.....3.0 t per m (iii) 10.....20 t per m solid 10.....50 t per m floating	No specification.	Each case is considered on merits and no standard formulae are used.	
		Minimum headroom provided (a) overall roads-16 ft 6 in. (b) In pedestrian subways-7 ft. (c) In cycle or combined cycle and pedestrian subways-7 ft 6 in. (d) In cattle creeps-8 ft For detailed information refer B.S. 153—Girder Bridges Part 3 loads-stresses, Section—A loads, and Ministry of Transport Memorandum No. 771.	kg per cm ² multiplied by the area containing of the width of the pier and the thickness of ice.

QUESTIONS	INDIA	ITALY	JAPAN
11. Foot bridges. (i) The minimum width of foot bridge according to table. (ii) The loading specified in the design of deck in (a) Urban areas (b) Rural areas (iii) Loading stipulated for the design hand-rails.	Clause 116* Shall be designed to resist a lateral horizontal force and a vertical force, each of 100 lb per linear foot applied simultaneously at the top.	No details given.	(i) 1.5 m (ii) For urban and rural areas. (a) 500 kg per m ² for deck (b) 350 kg per m ² for main girder (iii) 250 kg per m
12. Any formulae stipulated for calculating the impact on piers and abutments to floating objects in the river (i) floating timber (ii) vessels and river craft (iii) ice	No details given.	No details given.	(i) (ii) (iii) (iv) Car—[100 t in car direction, 50 t in other directions at height of 1.2 m]
13. Any other information supplied.			

*Articles referred

*Clause referred to relates to the Indian Roads Congress Standard Specifications and Code of Practice for Road Bridges—Sections I & II (1964).

clauses referred to
Road Authorities

Transport-Communications Monthly Review

	MALAYSIA	NEW SOUTH WALES	NEW ZEALAND	TURKEY
<p>N</p> <p>and rural</p> <p>per m² for</p> <p>per m² for</p> <p>irder</p> <p>er m</p>	<p>(i) 6 ft</p> <p>(a) 100 lb per ft</p> <p>(b) —do—</p> <p>25-100 lb per linear ft</p> <p>ause 15, B.S. 153—Part 3A:1954</p>	<p>Clause 2.12*</p> <p>(i) not given</p> <p>(ii) same as Q. 10(i) & (ii)</p> <p>(iii) Top members of railings Lateral horizontal force of 150 lb per linear ft and simultaneous vertical force of 100 lb per linear foot applied at top of railing. Lower railing: Lateral horizontal level force for 150 lb per linear ft.</p>	<p>(i) Minimum width between rails to be 6 ft.</p> <p>(ii)</p> <p>(a) Live load 100 lb per sq. ft.</p> <p>(b) Live load 60 lb per sq. ft. (except over motorways)</p> <p>(iii) Lateral load of 60 lb per linear ft applied at top rail level.</p>	<p>(i) 2.5 m</p> <p>(ii)</p> <p>(a) 400 kg per m²</p> <p>(b) 250 kg per m²</p> <p>(iii) 100 kg per m</p>
<p>[100 t in car on, 50 t in other ons at height of</p>	<p>Nil</p>	<p>Clause 2.17*</p> <p>(i) Force to be calculated on the assumptions that the log weighs 2 Tons and travels at normal stream velocity. The log shall be stopped in a distance of 1 foot for timber piers, 6 in. for column type piers and 3 in. for solid type concrete piers. Should fender piles or timber sheathing be placed upstream from the pier to absorb the energy of the blow, distances may be increased.</p> <p>(ii) No details given.</p> <p>(iii) —do—</p>	<p>Not taken into account.</p>	<p>(i) —</p> <p>(ii) —</p> <p>(iii) 30 kg per cm² multiplied by the area consisting of the width of the pier and the thickness of ice.</p>
<p>Specifications and</p>				

auses referred to relate to Highway Bridge Design Specifications of N.A. of A.S. Road Authorities (1965).

QUESTIONS	Clau	NORWAY	PHILIPPINES	RHODESIA
<p>11. Foot bridges.</p> <p>(i) The minimum width of foot bridge acceptable.</p> <p>(ii) The loading specified in the design of deck in</p> <p>(a) Urban areas</p> <p>(b) Rural areas</p> <p>(iii) Loading stipulated for the design hand-rails.</p>	<p>Sl resist force each foot at the</p>	<p>So far no specifications adopted.</p>	<p>(i) 5 ft (clear roadway)</p> <p>(ii)</p> <p>(a) 100 lb per sq. ft.</p> <p>(b) —do—</p> <p>(iii) 150 lb per sq. ft. horizontal force with simultaneous vertical force of 150 lb per sq. ft. applied at the top of railing.</p>	<p>(i) 2 ft 6 in. 8 ft, if combined cycle track bridge.</p> <p>(ii) Section 4 C, 153/1954 Part 3 tion A</p> <p>(a) 50 lb per sq. ft.</p> <p>(b) —do—</p> <p>(iii) 500 lb horizontal at 2 ft 6 in. surface level.</p>
<p>12. Any formulae stated for calculating the impact on piers and abutments to floating objects in the river</p> <p>(i) floating timber</p> <p>(ii) vessels and river craft</p> <p>(iii) ice</p>	Nc	<p>So far no specifications adopted.</p>	<p>Velocity of flowing water only considered.</p> <p>$P = KV^2$ where</p> <p>V = Velocity of water in ft per sec.</p> <p>K = a constant which is $1\frac{1}{2}$ for square ends, $1\frac{1}{2}$ for angle end where the angle is 30 degrees or less and $2\frac{2}{3}$ for circular pier.</p> <p>P = pressure in lb per sq. ft.</p>	<p>(i) No particular formula adopted. Each structure treated according to the vegetation predominant in catchment area, heavy large excessive bush, etc.</p> <p>(ii) N/A</p> <p>(iii) N/A</p>
<p>13. Any other information supplied.</p>				

*Articles referred

*Classification Code

December 19

RHODESIA

t 6 in.
t, if combined w
le track bridge.

ction 4 C, 1
3/1954 Part 3
on A

50 lb per sq. ft.

—do—

00 lb horizontal
t 2 ft 6 in. a
urface level.

No particular form
adopted. Each
ture treated acco
to the vegetation
predominant in
catchment area,
heavy large
excessive bush, et

) N/A

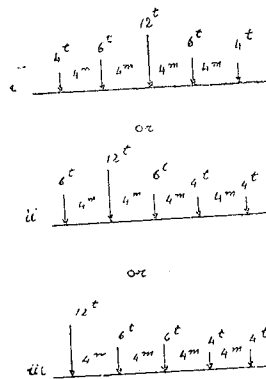
i) N/A

SWEDEN	SWITZERLAND	TURKEY
<p>(i) 2.5 m</p> <p>(ii) 400 kg per m². In special case, the load may be reduced to 250 kg per m².</p> <p>(iii) Transverse live load of 100 kg per m applied at the top of railing.</p>	<p>(i) Not prescribed.</p> <p>(ii) 360 kg per m² and one over load of one t.</p> <p>(iii) 120 kg per m in towns 80 kg per m outside the towns.</p>	<p>(i) 2.5 m</p> <p>(ii)</p> <p>(a) 400 kg per m²</p> <p>(b) 250 kg per m²</p> <p>(iii) 100 kg per m</p>
<p>(i) Nil</p> <p>(ii) Nil</p> <p>(iii) Between 10 and 20 t per m of abutment or pier in question. In flowing water with ice, block pressure parallel to the stream may be assumed between 0.5 to 1.5 t per m of span length and 1/5th thereof perpendicular to the stream.</p>	<p>Not prescribed</p>	<p>(i) ———</p> <p>(ii) ———</p> <p>(iii) 30 kg per cm² multiplied by the area consisting of the width of the pier and the thickness of ice.</p>
<p>Vertical clearance</p> <p>(i) Roadway 4.6 m</p> <p>(ii) Cycle track 2.5 m</p> <p>(iii) Foot-path 2.2 m</p>		

BELGIUM

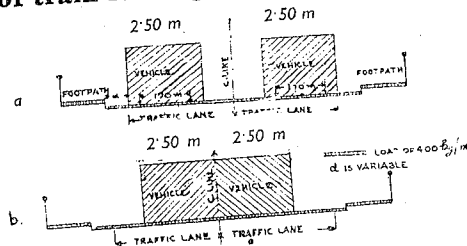
6. Ground Slab

1. Normal train loading



Over traffic lane 2.50 m² minimum wide to 4 m maximum wide and simultaneously a load of 400 kg per m² uniformly distributed on the carriageways and footpaths.

2. Lateral disposition of train loading



3. Number of train loadings

One train loading over traffic lane plus a load of 400 kg per m².

4. Effective wind pressure

The area of the train of vehicles assumed in elevation is a rectangular screen 2 m high with a length equal to the length of the train.

5. Impact factor

The impact factor due to live loads is the same for in question No. 1—i, ii, iii; and is given by the following formula :

$$\phi = 1 + 0.377 \frac{v}{\sqrt{l\alpha}} \sqrt{1 + \frac{2Q}{P}}$$

where

v = speed in kilometre per hour, always greater than 60

l = distance between supports, in metre

$$\alpha = \frac{l}{f_s}$$

f_s = static deflection, in metre, due to dead weight

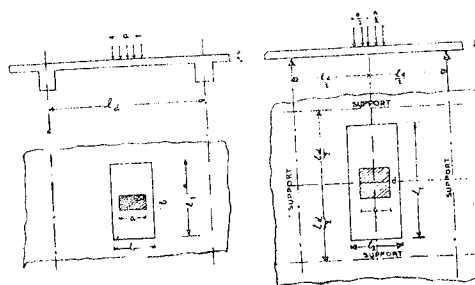
Q = moving loads on the bridge deck, in tonnes

P = deadweight of the bridge, in tonnes

6. Ground contact area

Slab bearing in one direction

Slab bearing on four sides



$$l_1 = b + 2h_0 + \frac{ld}{3}$$

$$l_2 = a + 2h_0$$

$$l_1 = a + 2h_0 + \frac{ld}{2.5}$$

$$l_2 = a + 2h_0$$

ld = span of the slab
 h_0 = width of the slab

7. Equivalent U.D.L.

There are no regulations.

8. Braking force

The braking force is equal to 1/20 of the load 400 kg per m² (without impact factor) distributed on the entire surface of the bridge deck or, if it is more unfavourable, 3/10 of the wheel loads placed on the deck.

9. Surcharge effect

The surcharge effect considered in the design of abutments of the bridge is equal to the live loads considered for calculating the bridge.

10. Footpath loading

crowd load = 400 kg per m² with impact factor

minimum width : 1 metre

special loading : 1000 kg situated at 40 cm from the hand-rails.

11. Footbridges

(i) no regulations

(ii) loading 400 kg per m² for urban and rural areas

(iii) a horizontal and transverse load of 100 kg per metre run on the hand-rails.

12. Impact due to floating objects

(i) no regulations

(ii) the impact is equal to 5% of the weight of vessels and it is assumed to have an influence 1 m above the water level.

(iii) no regulations.

TABLE 1
TABLE OF EQUIVALENT HEIGHTS (H) OF SURCHARGE OF
EARTH

Depth of abutment below the road level in feet	H in feet for the concentrated surface loads due to the wheel or track loads of the following I.R.C. Standard Loadings					
	I.R.C. CLASS AA LOADING		I.R.C. CLASS A LOADING		I.R.C. CLASS B LOADING	
	Single-lane bridges	Multi-lane bridges	Single-lane bridges	Multi-lane bridges	Single-lane bridges	Multi-lane bridges
1	84.6	50.8	46.8	56.4	28.2	33.8
2	65.3	39.2	36.1	43.5	21.8	26.1
3	51.6	31.0	28.6	34.4	17.2	20.6
4	42.9	25.7	23.7	28.6	14.3	17.2
5	36.9	22.1	20.4	24.6	12.3	14.8
6	32.6	19.5	18.0	21.7	10.9	13.0
7	29.1	17.5	16.1	19.4	9.7	11.6
8	26.4	15.8	14.6	17.6	8.8	10.6
9	24.1	14.5	13.4	16.1	8.1	9.7
10	22.4	13.4	12.4	14.9	7.5	8.9
12	19.4	11.6	10.7	12.9	6.5	7.7
14	17.1	10.3	9.5	11.4	5.7	6.8
15	16.2	9.7	9.0	10.8	5.4	6.5
16	15.3	9.2	8.5	10.2	5.1	6.1
18	13.8	8.3	7.6	9.2	4.6	5.5
20	12.6	7.6	7.0	8.4	4.2	5.0
22	11.6	6.9	6.4	7.7	3.9	4.6
24	10.8	6.5	6.0	7.2	3.6	4.3
25	10.4	6.2	5.7	6.9	3.5	4.1
26	10.1	6.0	5.6	6.7	3.4	4.0
28	9.5	5.7	5.2	6.3	3.2	3.8
30	8.9	5.3	4.9	5.9	3.0	3.5
32	8.4	5.0	4.6	5.6	2.8	3.4
and above						

Note :—The above figures are based on the following values for the constants for the abutments and the backfill :

- (1) Length of abutment (L) = 15 ft for single-lane bridges and 25 ft for multi-lane bridges.
 - (2) Angle of internal friction of the backfill (ϕ) = 30°
 - (3) Wt of backfill (W).....100 lb per cu. ft.
 - (4) The resultant earth pressure acts in a horizontal direction.
- For different values, say, L_1 , ϕ_1 and W_1 for the constants, the figures given in the above Table should be multiplied by the following factors :

$$\frac{L \text{ (15 or 25 as the case may be)}}{L_1} \cdot \frac{(1 + \sin \phi_1)}{3(1 - \sin \phi_1)} \text{ and } \frac{100}{W_1} \text{ respectively.}$$

TABLE 2—Equivalent Distributed Load
(From Bridge Loadings—Italy)

Bending load in t/ml						Shear load in t/ml					
Span m	Civil Loading		Military Loading			Span m	Civil Loading		Military Loading		
	Type	Type	Type	Type	Type		Type	Type	Type	Type	
	1	2	4	5	6		1	2	4	5	6
1	16.000	24.000	28.000	10.667	38.000	1	16.000	24.000	28.000	12.160	36.000
1.5	10.667	16.000	18.667	9.027	25.333	1.5	10.667	16.000	24.640	10.287	31.680
2	8.000	12.000	15.541	8.188	19.981	2	8.000	12.000	20.860	9.787	26.820
2.5	6.400	9.600	14.193	7.285	18.248	2.5	6.400	9.600	17.830	9.114	22.925
3	5.333	8.000	12.859	7.329	16.534	3	5.333	8.000	15.493	8.770	19.920
3.5	4.571	6.857	11.677	7.561	15.013	3.5	4.898	7.347	13.669	8.620	17.574
4	4.000	6.000	10.658	7.455	13.703	4	4.500	6.750	12.215	8.400	15.705
4.5	3.556	5.333	9.784	7.252	12.579	4.5	4.148	6.222	11.034	8.217	14.187
5	3.200	4.800	9.032	7.143	11.612	5	3.840	5.760	10.058	7.936	12.931
5.5	2.921	4.382	8.381	6.954	10.776	5.5	3.570	5.355	9.497	7.617	12.229
6	2.778	4.167	7.814	6.727	10.047	6	3.333	5.000	9.147	7.289	11.803
6.5	2.651	3.965	7.316	6.486	9.407	6.5	3.314	4.686	8.788	6.968	11.359
7	2.612	3.778	6.877	6.243	8.842	7	3.265	4.408	8.434	6.661	10.917
7.5	2.560	3.605	6.486	6.005	8.339	7.5	3.200	4.160	8.094	6.372	10.487
8	2.500	3.445	6.136	5.776	7.889	8	3.125	3.938	7.770	6.133	10.077
8.5	2.436	3.297	5.990	5.559	7.728	8.5	3.045	3.737	7.464	5.950	9.687
9	2.370	3.160	5.854	5.352	7.561	9	2.963	3.556	7.258	5.813	9.401
9.5	2.305	3.031	5.713	5.157	7.387	9.5	2.925	3.391	7.057	5.717	9.125
10	2.240	2.916	5.599	4.974	7.209	10	2.880	3.240	6.877	5.639	8.873
11	2.128	2.705	5.411	4.639	6.857	11	2.777	2.975	6.609	5.554	8.474
12	2.111	2.521	5.206	4.342	6.518	12	2.667	2.750	6.331	5.511	8.079
13	2.083	2.360	4.997	4.231	6.200	13	2.651	2.556	6.057	5.453	7.700
14	2.122	2.217	4.793	4.137	5.933	14	2.612	2.388	5.809	5.355	7.358
15	2.133	2.091	4.597	4.110	5.710	15	2.560	2.240	5.607	5.234	7.072
16	2.125	1.978	4.420	4.106	5.521	16	2.531	2.109	5.408	5.117	6.798
17	2.104	1.876	4.340	4.120	5.357	17	2.491	1.993	5.216	5.032	6.537
18	2.086	1.784	4.291	4.136	5.195	18	2.444	1.889	5.098	4.982	6.291
19	2.083	1.701	4.230	4.204	5.038	19	2.438	1.795	5.051	4.958	6.059
20	2.080	1.625	4.159	4.253	4.885	20	2.420	1.710	5.006	4.944	5.840
21	2.068	1.555	4.113	4.293	4.739	21	2.395	1.633	4.947	4.920	5.635
22	2.050	1.491	4.060	4.308	4.599	22	2.380	1.562	4.877	4.879	5.443
23	2.028	1.432	4.000	4.304	4.475	23	2.359	1.497	4.829	4.827	5.261
24	2.028	1.378	3.936	4.286	4.363	24	2.333	1.438	4.795	4.778	5.091
25	2.022	1.327	3.870	4.257	4.255	25	2.330	1.382	4.750	4.742	4.930
26	2.036	1.280	3.818	4.220	4.151	26	2.320	1.331	4.702	4.720	4.778
27	2.041	1.236	3.790	4.177	4.051	27	2.305	1.284	4.664	4.709	4.635
28	2.041	1.196	3.766	4.145	3.954	28	2.296	1.240	4.634	4.702	4.500
29	2.036	1.157	3.737	4.118	3.860	29	2.283	1.199	4.600	4.688	4.372
30	2.031	1.121	3.703	4.106	3.771	30	2.267	1.160	4.559	4.665	4.251
31	2.031	1.088	3.666	4.104	3.685	31	2.264	1.124	4.515	4.635	4.137
32	2.031	1.056	3.626	4.105	3.602	32	2.258	1.090	4.477	4.608	4.028
33	2.028	1.026	3.584	4.110	3.522	33	2.248	1.058	4.436	4.591	3.924
34	2.021	0.997	3.567	4.125	3.446	34	2.242	1.028	4.391	4.578	3.825
35	2.012	0.971	3.567	4.137	3.372	35	2.233	0.999	4.357	4.571	3.732
36	2.012	0.945	3.583	4.157	3.302	36	2.222	0.972	4.340	4.568	3.642
37	2.010	0.921	3.612	4.169	3.233	37	2.221	0.947	4.330	4.558	3.557
38	2.017	0.898	3.634	4.174	3.168	38	2.216	0.922	4.314	4.543	3.475
39	2.020	0.876	3.648	4.173	3.105	39	2.209	0.899	4.294	4.523	3.397
40	2.020	0.856	3.657	4.167	3.044	40	2.205	0.878	4.276	4.508	3.323
45	2.015	0.765	3.673	4.107	2.771	45	2.181	0.782	4.212	4.475	2.993
50	2.010	0.691	3.658	4.116	2.541	50	2.163	0.706	4.135	4.429	2.722
55	2.010	0.631	3.646	4.138	2.345	55	2.147	0.643	4.079	4.400	2.496
60	2.004	0.580	3.621	4.107	2.177	60	2.133	0.590	4.041	4.380	2.304
70	2.005	0.500	3.570	4.124	1.903	70	2.116	0.507	3.962	4.337	1.997
80	2.005	0.439	3.602	4.104	1.689	80	2.101	0.444	3.923	4.306	1.762
90	2.003	0.391	3.597	4.107	1.518	90	2.089	0.396	3.878	4.286	1.576
100	2.003	0.353	3.577	4.112	1.379	100	2.079	0.356	3.849	4.268	1.426
120	2.001	0.295	3.583	4.107	1.164	120	2.061	0.298	3.799	4.239	1.197
140	2.002	0.253	3.570	4.103	1.007	140	2.051	0.255	3.765	4.220	1.031
160	2.001	0.222	3.577	4.104	0.887	160	2.043	0.224	3.741	4.205	0.906
180	2.000	0.198	3.572	4.106	0.793	180	2.037	0.199	3.722	4.190	0.808
200	2.001	0.178	3.571	4.086	0.717	200	2.033	0.179	3.707	4.179	0.729

Interpolation is to be made.

131

N.B. For any intermediate span, linear interpolation is to be made.

UNITED STATES OF AMERICA

PLATE I

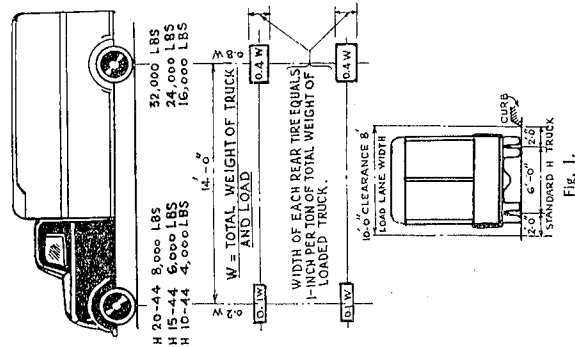


Fig. 1.

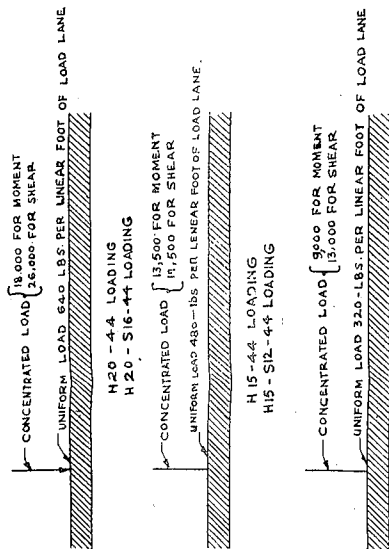


Fig. 2.

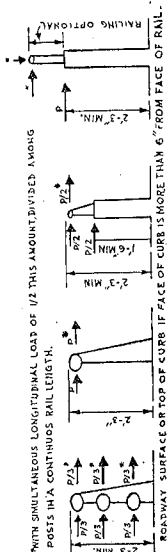


Fig. 3.

TRAFFIC RAILING

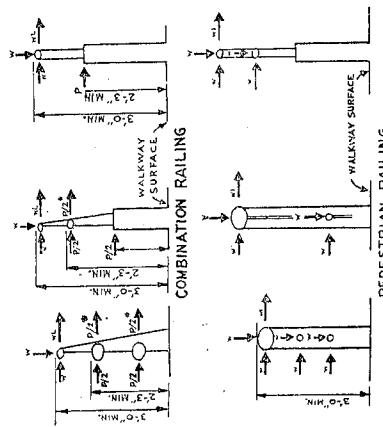


Fig. 4.

Military loading for bridges on the Interstate System

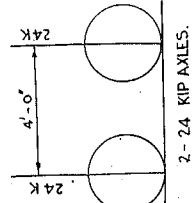


Fig. 5.

LEGEND
P=10,000 lb. L=Post Spacing for Traffic Railing, W=50 lb P.L.F. I=Post Spacing for Pedestrian Railing. Rail Load shown Left, Post Load shown Right.
Note: The shapes of Rail Members are illustrative only. Any Material or combination of Materials listed in Article 1.1.10(A) may be used in any Configuration

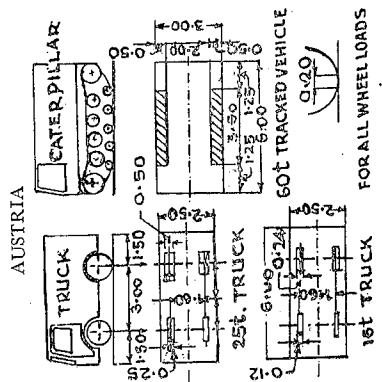


Fig. 6.

FEDERAL REPUBLIC OF GERMANY

Dimensions of Design Vehicle (in Metre)
Truck (LKW)

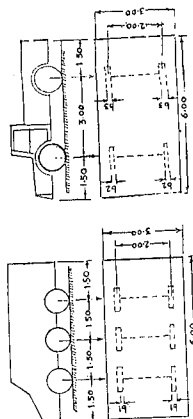


Table 2. Load and Contact Width of Design Vehicle

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 3. Intermediate Class (only for Checking the Existing Bridges)

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 4. Main Lane considered in Calculation Width=3m

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 5. Design Load for Intermediate Class (only for Checking the Existing Bridges)

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 6. Gross Girders both up to a spacing of 2.0m

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 7. Gross Girders both up to a spacing of 2.0m

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 8. Gross Girders both up to a spacing of 2.0m

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

Table 9. Gross Girders both up to a spacing of 2.0m

Class	HEAVY TRUCKS (SLW)									
	1	2	3	4	5	6	7	8	9	10
Total Load	60	60	60	60	60	60	60	60	60	60
Wheel Load	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Width of Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Width of Wheel Contact	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60
Wheel Contact Load	16	16	16	16	16	16	16	16	16	16
Wheel Contact Area of each	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24	0.24

GREAT BRITAIN

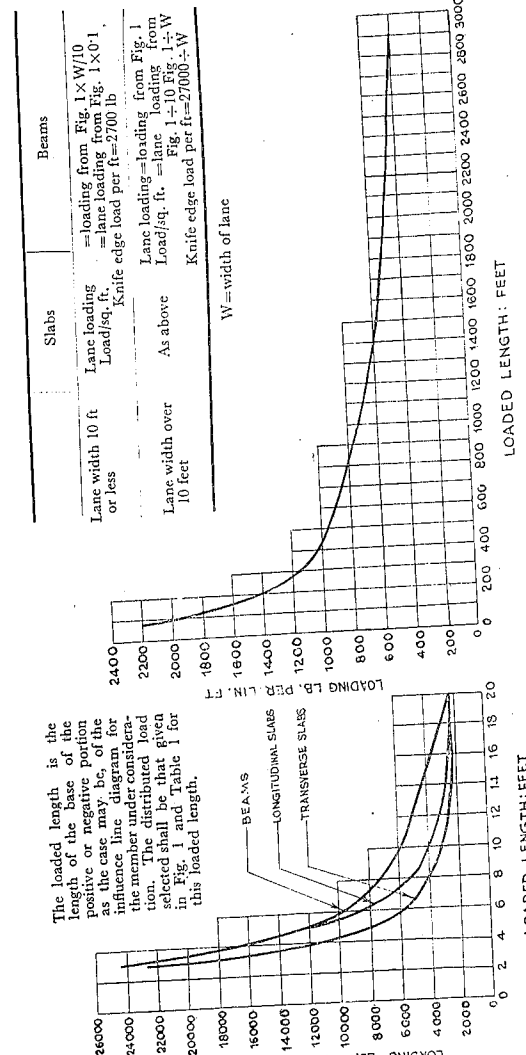
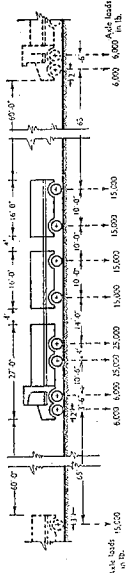


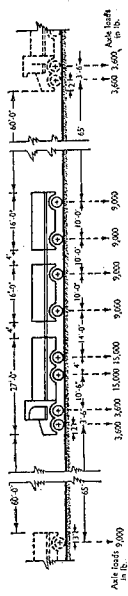
Fig. 8.

Figs. and Tables referred to above relate to British Standard Highway Loadings, B.S. 153 : Part 3, Section A—1954

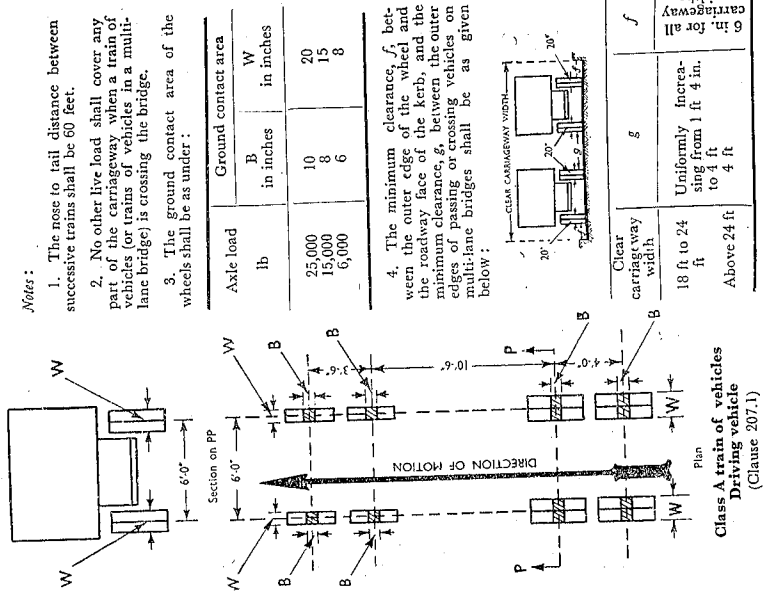
INDIAN ROADS CONGRESS BRIDGE LOADINGS



Class A train of vehicles



Class B Train of Vehicles



Notes:

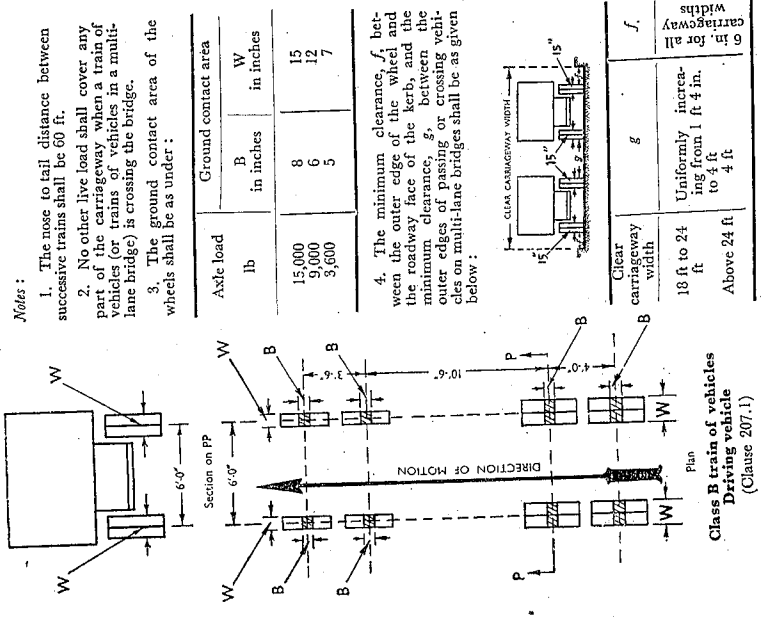
1. The nose to tail distance between successive trains shall be 60 feet.
2. No other live load shall cover any part of the carriageway when a train of vehicles (or trains of vehicles in a multi-lane bridge) is crossing the bridge.
3. The ground contact area of the wheels shall be as under:

Axle load lb	Ground contact area	
	B in inches	W in inches
25,000	10	20
15,000	8	15
9,000	6	8

4. The minimum clearance, f , between the outer edge of the wheel and the roadway face of the kerb, and the minimum clearance, g , between the outer edges of passing or crossing vehicles on multi-lane bridges shall be as given below:

Clear carriageway width	g	f 6 in. for all carriageway widths
18 ft to 24 ft	Uniformly increasing from 1 ft 4 in. to 4 ft	
Above 24 ft	4 ft	

Class A train of vehicles
Driving vehicle
(Clause 207.1)



Notes:

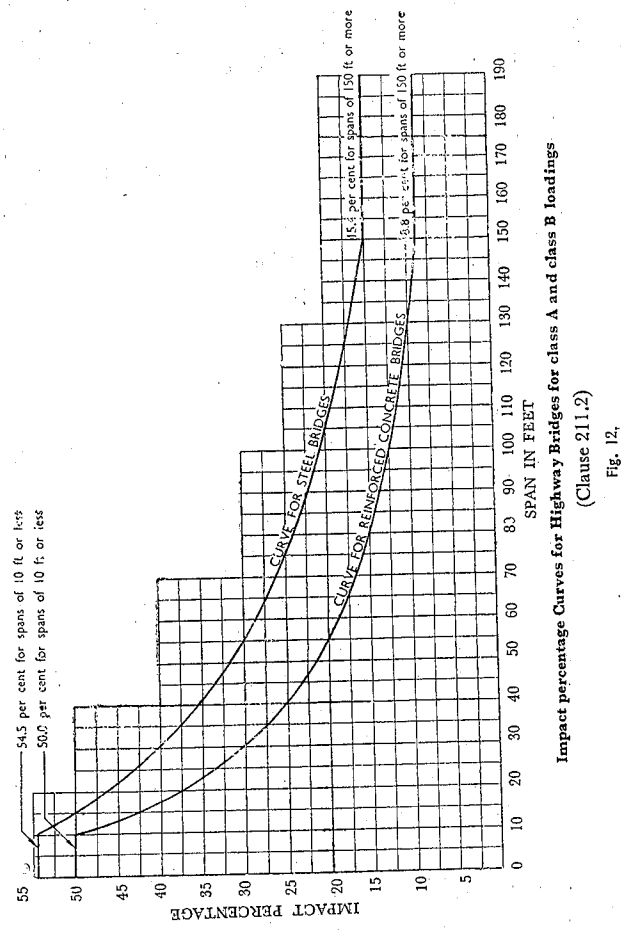
1. The nose to tail distance between successive trains shall be 60 ft.
2. No other live load shall cover any part of the carriageway when a train of vehicles (or trains of vehicles in a multi-lane bridge) is crossing the bridge.
3. The ground contact area of the wheels shall be as under:

Axle load lb	Ground contact area	
	B in inches	W in inches
15,000	8	15
9,000	6	12
3,600	5	7

4. The minimum clearance, f , between the outer edge of the wheel and the roadway face of the kerb, and the minimum clearance, g , between the outer edges of passing or crossing vehicles on multi-lane bridges shall be as given below:

Clear carriageway width	g	f 6 in. for all carriageway widths
18 ft to 24 ft	Uniformly increasing from 1 ft 4 in. to 4 ft	
Above 24 ft	4 ft	

Class B train of vehicles
Driving vehicle
(Clause 207.1)

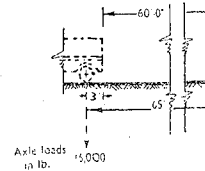
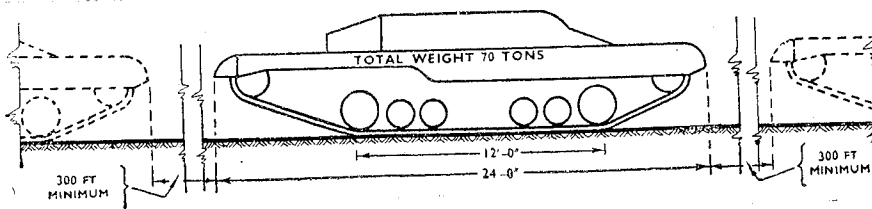


Impact Percentage Curves for Highway Bridges for class A and class B loadings.
(Clause 211.2)

Fig. 12.

Fig. 10.

Fig. 11.



Notes :

1. The nose to tail spacing between two successive vehicles shall not be less than 300 ft.

2. For multi-lane bridges and culverts, one train of Class AA tracked or wheeled vehicles, whichever creates severer conditions shall be considered for every two-traffic-lane width.

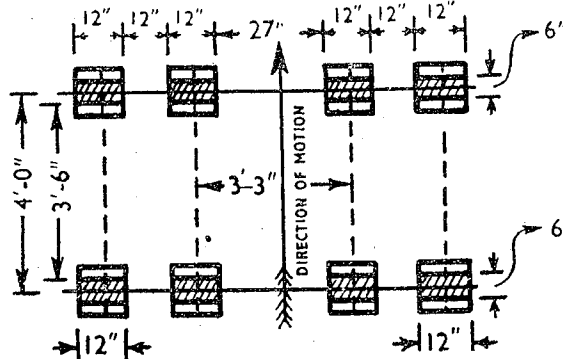
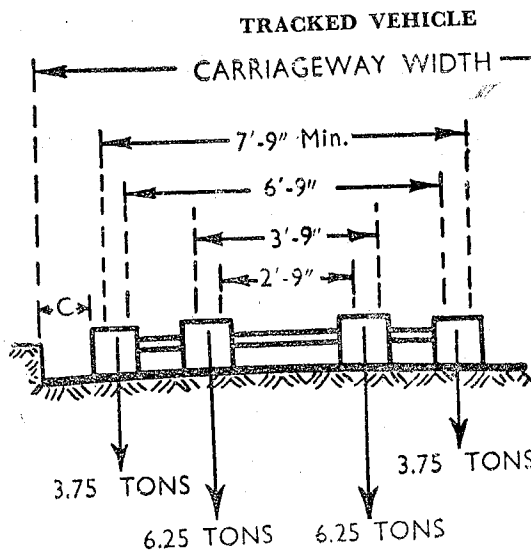
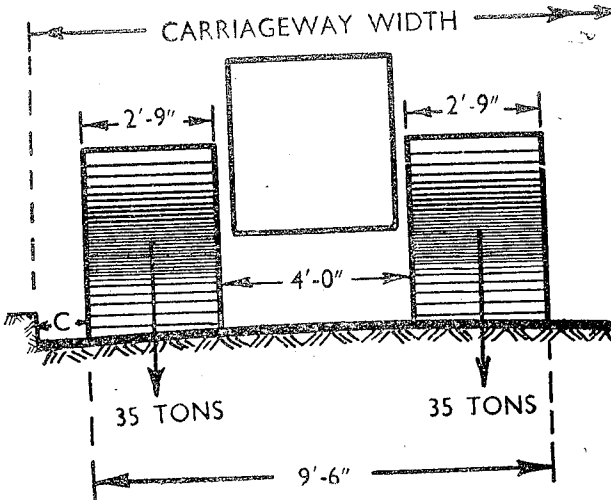
No other live load shall be considered on any part of the said 2-lane width carriageway of the bridge when the above mentioned train of vehicles is crossing the bridge.

3. The maximum loads for the wheeled vehicle shall be

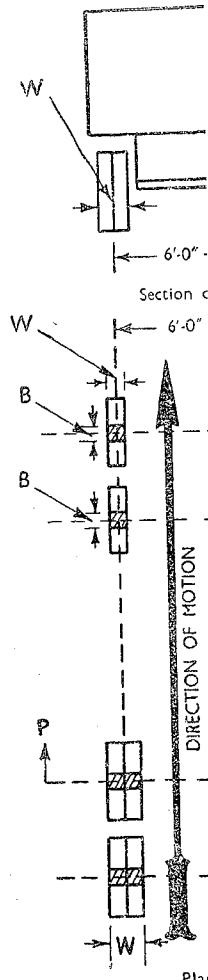
20 tons for a single axle, or 40 tons for a bogie of two axles spaced not more than 40 ft centres.

4. The minimum clearance between the road face of the kerb and the outer edge of the wheel or track, C, shall be as under :

Carriageway Width	Minimum Value of C
Single Lane Bridges	
12 ft and above	1 ft-0 in.
Multiple Lane Bridges	
Less than 18 ft	2 ft-0 in.
18 ft or above	4 ft-0 in.



Plan : WHEELED VEHICLE
Class AA tracked wheeled vehicles (Clause 207-1)
Fig. 9.



Plan
Class AA train
Driving
(Clause 207-1)

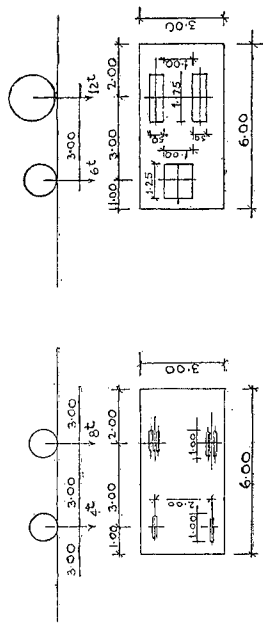
W/10
1x0.1

m Fig. 1
ng from
Fig. 1 ÷ W
100 ÷ W

2800 3000.

ITALY
CIVIL LOADING

Type 1
CONTINUOUS TRAIN OF 12t TRUCK
Type 2
SINGLE 18t STEAM ROLLER

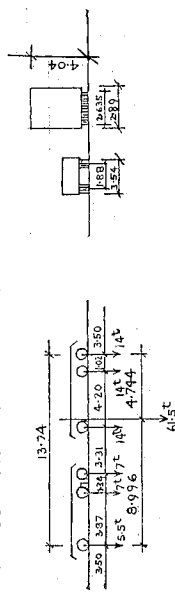


Type 3

CROWD LOAD OF 400 kg/m

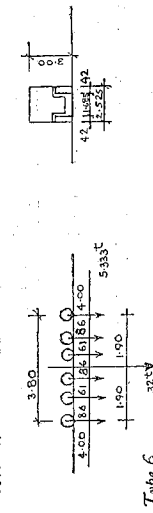
Type 4

CONTINUOUS TRAIN OF MILITARY LOAD OF 61.5t



Type 5

CONTINUOUS TRAIN OF MILITARY LOAD OF 32t



Type 6

SINGLE MILITARY LOAD OF 74.5t

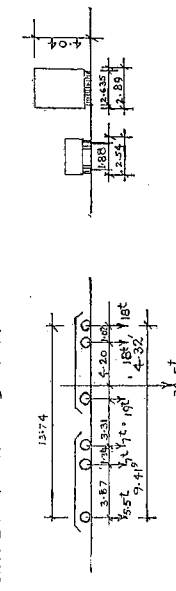


Fig. 13

PHILIPPINES

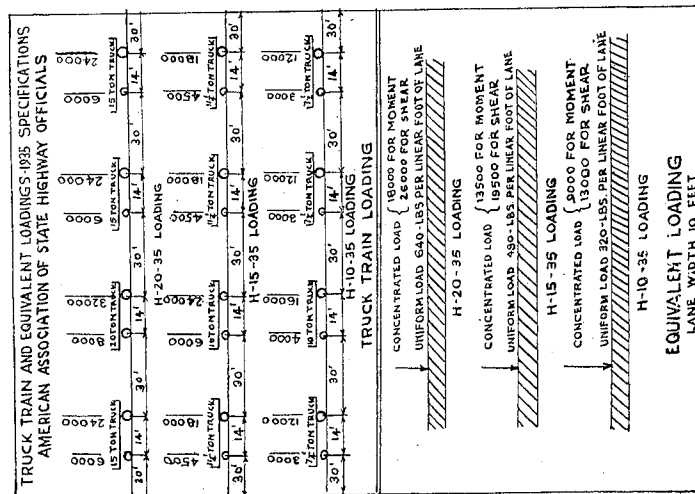


Fig. 14.

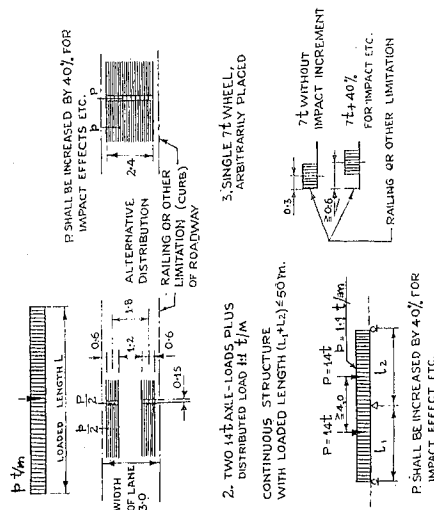
SWEDEN

TRAFFIC LOADING FOR BRIDGES

Cl. Lane Loading

1. One=14t Axle-Load Plus Distributed Load $P \frac{t}{m}$.

$P=14t$
 $L < 10$ $p=2.4$
 $L=10-90$ $p=2.4 - \frac{13(L-10)}{80}$
 $L > 90$ $p=1.1$



b. SINGLE-TRUCK LOADING

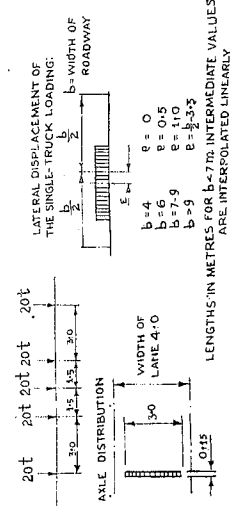
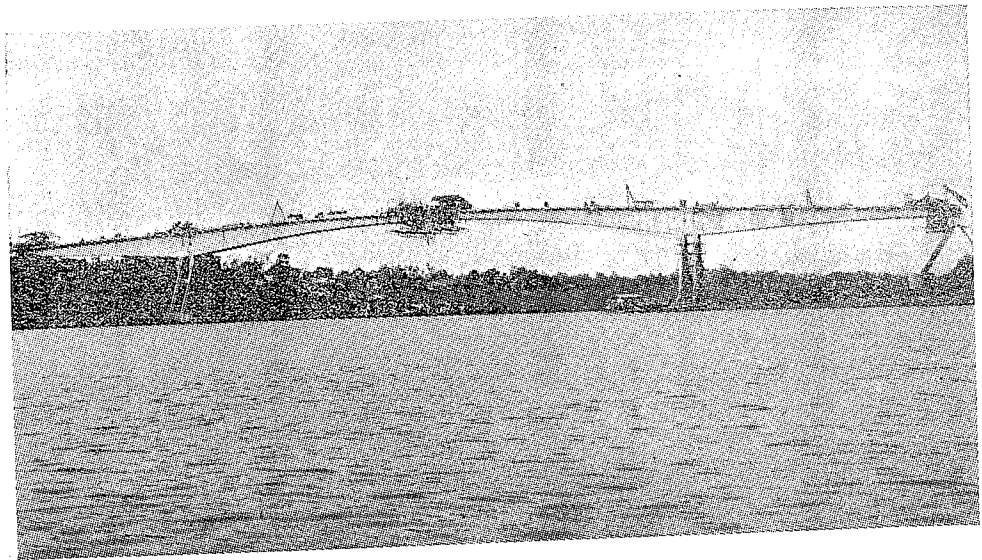


Fig. 15.



BARAK BRIDGE

When completed this bridge will have the largest prestressed span of any bridge so far built in India.

The bridge will be 924 ft. long with deckings of 77 ft.—185 ft.—400 ft.—185 ft.—77 ft. span, providing a 24 ft. wide roadway. This “Cantilever” in-situ construction was done without staging, which ensured navigation during construction.

Prestressing has been done by the Freyssinet method.

Designed and constructed to the orders of the Chief Engineer (Roads) P.W.D., Shillong, Assam.

By

GAMMON INDIA LIMITED

Civil Engineers & Contractors

Gammon House, Prabhadevi, Cadell Road,

BOMBAY - 28 DD.

Telephones : { 454261 (5 lines)
452214 - 452215

Telegrams : { “Gammon”, Bombay
Dadar.

Telephone : 25 31 16
45 39 49

Telegram : JOSHENGER
BOMBAY.

S. B. JOSHI & CO. LTD.

STRUCTURAL ENGINEERS & CONTRACTORS,

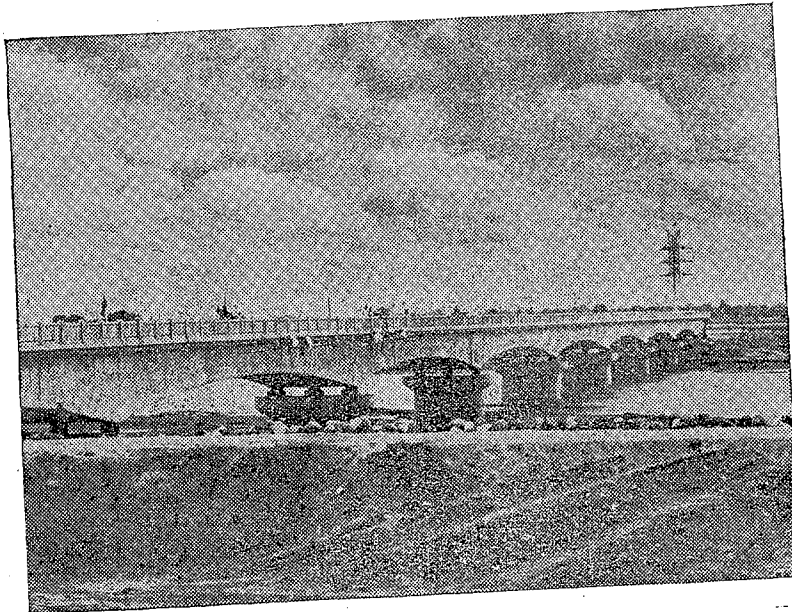
Registered Office :

Examiner Press Building,

35, Dalal Street,

Fort, BOMBAY-1.

We undertake design and construction of major Civil Engineering works including R. C. C. and Prestressed Bridges, Factories, Tunnels, Dams and Marine Works.



KOSI BRIDGE AT RAMPUR (U.P.)

Supported on 81' Deep Well Foundations R.C.C. Box Girder Balanced Cantilever Type Bridge over River Kosi at Rampur on National Highway No. 24 was Completed in July 1965 in Just 15 Working Months—Length: 1055'-0" Span: 135'-0"

Regional Offices : Cuttack (Orissa), Lucknow (U.P.) and Patna (Bihar).

ALLWYN

Proudly announce the commencement of manufacturers of :

*TIPPING HOISTS AND POWER TAKE OFF WITH A CAPACITY OF 7 TO 12 TONS TO SUIT TATA MERCEDEZ BENZ, DODGE, BEDFORD AND LEYLAND COMET CHASSIS.

TIPPERS TO BE ONE WAY REAR TIPPING. MAXIMUM TIPPING ANGLE 50°

*TRAILERS OF CAPACITY 12 AND 25 TONS TO SUIT DIFFERENT REQUIREMENTS.

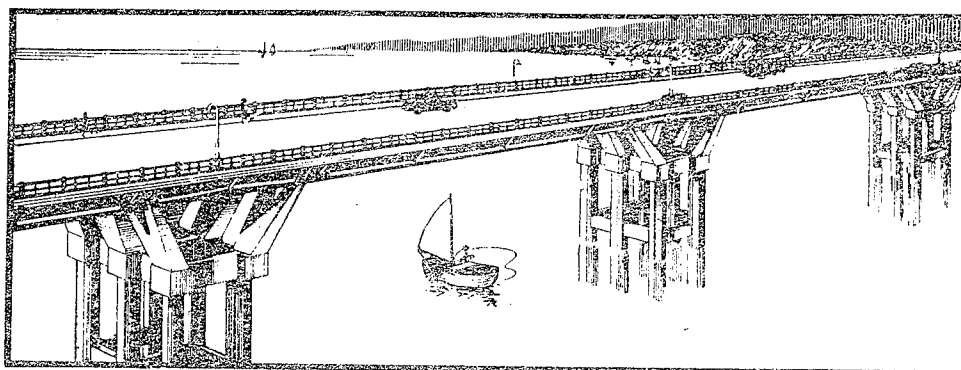
Please address your enquiries to : -

The Hyderabad Allwyn Metal Works Ltd.,

Auto Division, Sanatnagar,

Hyderabad-18 (A.P.)

FREYSSINET, ONCE AGAIN



An artist's impression of the Thana Creek Bridge

It is quite a job to hold together the enormous mass of cement concrete that goes into the construction of a bridge—especially more so when the bridge is the one now under construction across the Thana Creek to link Greater Bombay with a vast area of open land in Panvel. This road bridge, which is estimated to cost Rs. 236,00,000 will be 6,015 feet long and 49 ft. wide, and will have its central span as long as 175 ft. Engineers at Gammon India Limited, the designers of the bridge, specified the Freyssinet system for carrying out the task of prestressing, depending once again on the proven qualities of this system—ECONOMY and DEPENDABILITY—testified by the large number of structures built throughout India for more than 20 years now, using the Freyssinet Prestressing equipment and techniques.

For further information and free advice on Prestressed Concrete contact :

THE FREYSSINET PRESTRESSED CONCRETE CO., LTD.

(Agents of Messrs. S. T. U. P.—PARIS)

120, Vaswani Mansion, Dinsha Vachha Road, Churchgate Reclamation, Bombay-I.

Telephone : 245920

"Grams : FREYSSI" Bombay.

Printed at the R. K. Printers, Kamla Nagar, and Edited & published by S. Hukam Singh, Technical Secretary, Indian Roads Congress, Jamnagar House, Shahjahan Road, New Delhi-II—5000.